

表録

運輸省港湾技術研究所

# 港湾技術研究所 報告

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REPORT OF  
THE PORT AND HARBOUR RESEARCH  
INSTITUTE  
MINISTRY OF TRANSPORT

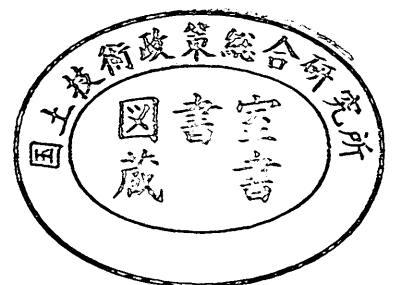
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# 港湾技術研究所報告 (REPORT OF P.H.R.I.)

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## 1. An Experimental Study of the Turbulent Structure of Wind over Water Waves

Hajime KATO\*  
Kikuo SANO\*\*

### Synopsis

By the use of an X-array hot-wire system and the digital method, the measurements have been made of the horizontal and vertical wind velocity fluctuations,  $u'$  and  $w'$ , over the three kind of waves in a wind-wave tunnel. The cross spectral analyses with respect to  $u'$  and  $w'$  were extensively used mainly to detect the wave-induced Reynolds stresses. In the first cases, where only mechanically generated long waves existed by the application of soap into water, the phase averaging were effectively used to obtain the wave-induced velocity components. When the wave height  $H$  was small ( $H=1.1$  cm), only the positive wave-induced Reynolds stresses were found at the small height above the wave crests. For the wave heights as large as  $H=6.0$  or  $7.0$  cm, however, the negative wave-induced stresses were prevailing over most of the height except the very small height above the crests where the stresses were positive. In the second cases of wind-waves only, the wave-induced velocity fluctuations also existed considerably at the lowermost measurement height. However they were almost  $90^\circ$  out of phase and did not make much contribution to the  $u'w'$ -cospectra, that is, to the total Reynolds stresses. Nevertheless the cospectra obtained in winter showed signs of the negative stress. Although the seasonal differences, not only quantitative but qualitative, in the values of  $-\overline{u'w'}$  were found, the results obtained at the seasons other than summer indicated that the turbulent Reynolds stresses are nearly constant over the short distances close above the wave crests. It was further found that the turbulent Reynolds stresses at those heights are in reasonably good agreement with the surface shear stresses estimated by applying the log-law to the lowest portion of the wind velocity profiles. In the third cases, where the larger mechanically generated waves and smaller wind waves coexisted, only the negative wave-induced Reynolds stresses of non-negligible magnitude were found regardless of the season, and the turbulent velocity fluctuations were predominant even at the lowermost measurement height somewhat in contrast to the first cases.

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# 1. 水面波上の風の乱流構造に関する実験的研究

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## 要 旨

2台の熱線風速計とX-型エレメントを使い、デジタルのデータ記録処理方式により、風洞水路において3種類の波の上での風速の水平、垂直変動成分  $u'$ ,  $w'$  の測定を行なった。主として、波により誘起されたレイノルズ応力 (wave-induced Reynolds stress) の存在を見わけるとの目的で、 $u'$  と  $w'$  についてのクロススペクトルを各ケースごとに求めた。石んを水中に入れて風波を消し、造波装置で起した長周期波だけのケースでは、波の各位置ごとに風速を平均することにより、波により誘起された速度変動成分が求められた。波高が 1.1cm と小さいときには、波により誘起されたレイノルズ応力は波の峯のすぐ上では正の値をとるが、高さと共にすぐ消失してしまうことがわかった。しかしながら波高が 6.0 または 7.0cm と大きいときには、波の峯の上の測定しうる最低の高さ（ここでは正になるようであるが）を除いたほとんどの高さで、はっきりと負の値をもっていた。風波だけのケースにおいては、波のすぐ上ではかなりの波によって誘起された速度変動成分があるが、それらはほとんど  $90^\circ$  近い位相差をもっていて、cospectra への、すなわちレイノルズ応力への寄与はあまり大きくなかった。それでも冬期の実験結果では、負の応力が多少ある傾向が見られた。 $-\overline{u'w'}$  の測定値には、定量的にも定性的にも季節ごとの差が存在したけれども、夏以外の季節にえられた測定結果からは、波のすぐ上の少しの高きで乱れのレイノルズ応力がほぼ一定に近いことがわかった。そしてそこでの値は、風速分布の水面に一番近い部分に対数法則をあてはめて求めた表面せん断力とだいたい一致した。石けんを使わず、したがって卓越した長周期波と小さな風波が共存する第3のケースでは、波によるレイノルズ応力は季節には関係なく、明らかに負の値のみを示し、また第1のケースとはやや対照的に、波のすぐ上の高さにおいても乱流変動成分の方がはるかに強力であった。

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## 1. Introduction

It is relatively recent years that the turbulent structures of wind over water waves were given attention in connection with the wave generation problem. Before the late 1950's the studies of wind in this context were conducted mainly in relation to the 'sheltering hypothesis' by Jeffreys (1924, 1925). Santon, Marshall & Houghton (1932) and Motzfeld (1937), respectively, made the measurements of air flow, especially the static pressure distribution, over the smooth solid wavy surface placed in a wind tunnel. Their results indicated that the pressure differences over those solid wave models are an order of magnitude smaller than the differences required by Jeffreys to account for the growth rate of observed waves. Although those results lowered the appraisal of the 'sheltering hypothesis' and led to a search for alternatives, these experiments were in fact almost irrelevant to the wave generation problem (Phillips, 1966).

In the late 1950's two wave generation theories were proposed by Phillips (1957) and Miles (1957), respectively. The former brought the turbulent character of wind into relief distinctly in relation to the wave generation, and the latter introduced the interaction between wind and waves in the inviscid wave generation theory. Miles (1957) discovered that a Reynolds stress is generated by the wave-induced air motion at the 'critical layer' where the wave speed equals the wind speed. This stress is then supported by the wave-induced velocity fluctuations in the air flow below the 'critical layer', and continually transfers momentum to the waves.

Since then not a few relevant experimental results have been published, mostly in favor of the inviscid theory by Miles for several years. Later on, however, it was found by Snyder & Cox (1966) and also Barnett & Wilkerson (1967) that the growth rate of waves observed in ocean is much greater than that predicted by the inviscid theory. In the meantime generalizing the inviscid theory, Phillips (1966) presented the detailed analytical discussions concerning the momentum transfer mechanism by the wave-induced turbulent air motion. Miles (1967) brought forward a conjecture that the significance of wave-induced perturbations in the turbulent Reynolds stresses for momentum transfer from wind to waves must increase with an appropriate scale parameter which is substantially larger in the field observations than the laboratory experiments. Miles also made it clear that the more detailed experimental data of those turbulent Reynolds stresses is a prerequisite to the further theoretical progress. Although Stewart (1967) submitted some doubts about the whole concept of Miles (1957) and alternatively proposed new ideas involving the effect of the turbulence in wind and of different flow configurations from that of Miles, the validity of those ideas could not be checked either for lack of the relevant wind data.

Even part from the connection with the wave generation problem, it is required from the engineering point of view to study the turbulent structures of wind over waves, especially in relation to the applicability of the so called log-law. That is, although the log-law has been well established for the wind profiles over the fixed rough surface, it will not be unfair to say that the relation has not yet been confirmed critically for the wind over the water waves. This is mainly because there is no means to measure the surface shear stress  $\tau_0$  so accurately as to be able to check the validity of the log-law. Nevertheless the log-

law has often been used in order to obtain the friction velocity  $u_*$  from the wind profiles, occasionally in conflicting ways.

The measurements of wind velocity spectra and Reynold stresses over the sea were made by Pond *et al.* (1963, 1966), Smith (1967) and Weiler & Burling (1967). Although these results were almost irrelevant to the wave generation problem, they contributed to shed light on the turbulent structures of wind up to several meters above the ocean waves.

In the laboratory Kato & Sano (1969) conducted the measurements of the wind velocity fluctuations over waves (with and without mechanically generated waves) by the use of a hot-wire and the digital method, and found that although there do exist the wave-induced velocity fluctuations in the degree in accordance with the wave conditions and the height above the wave crests, the turbulent fluctuations are predominant in general.

Recently Kendall (1970) measured the wind velocity fluctuations over the wavy wall made of a smooth neoprene rubber sheet which progressed upwind or down at controlled speed, and obtained the various wave-induced double correlation terms relevant to the wave generation theory. The results indicated that the waves strongly modulate the turbulent structure. The measurements of the horizontal and vertical velocity fluctuations over water waves were also made by Shemdin & Lai (1970) with and without mechanically generated waves in a wind-wave channel by the use of hot-film anemometers.

In this report we describe the primary results of an experimental study on the turbulent structure of wind and the wave-induced air motions over the three kind of waves in a wind-wave tunnel by the use of an X-array hot-wire system and the digital method similar to that described in the previous report (Kato & Sano, 1969).

## 2. Experimental Equipment and Procedure

### 2.1 General Description of Experiments

The present experiments were conducted in the same wind-wave tunnel as had been used in the previous experiments. The general sketch of the facility is again shown in Fig. 1. The uniform test section is 150 cm wide, 130 cm high and 2850 cm long. The side walls and most of the ceiling consist of glass plates. The enlarged part is situated on the leeward of the test section as seen in Fig. 1.

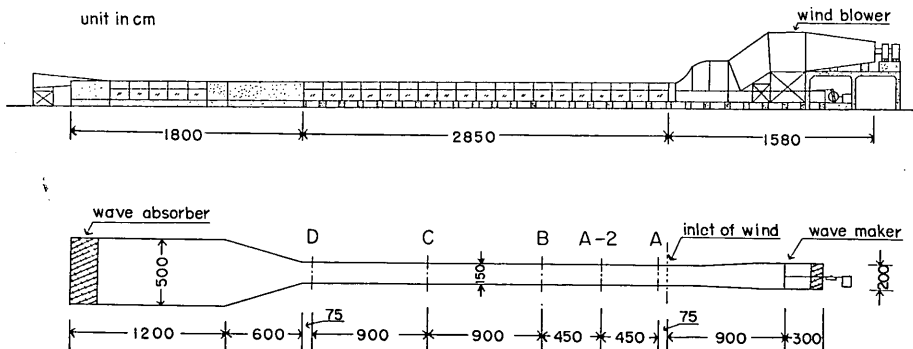


Fig. 1. Wind-Wave Tunnel



## An Experimental Study of the Turbulent Structure of Wind over Water Waves

At the leeward end of this part, wind is diffused and surface waves are dissipated by a slope of pebbles and stainless steel turnings. At the wind ward end of the waterway, a mechanical wave generator of piston type is equipped to generate regular waves of relatively long period. On the windward side of the test section is situated a wind blower, where wind is generated by an axial fan driven by a 50 KW variable-speed motor. Large scale turbulence and rotation of air flow caused by the fan are eliminated through guide vanes, a fine mesh screen and honeycombs. After passing the contraction section, air flow is supplied onto the water surface at the inlet section (shown in Fig. 1) by means of an adjustable-height transition plate of 3 m length which is hinged at the windward end. At the Section A, 75 cm leeward of the inlet, the wind speed distribution is quite uniform both vertically and horizontally and the turbulence level  $\sqrt{u'^2}/U$  ( $u'$  = streamwise velocity fluctuations,  $U$  = mean wind speed) is nearly 0.4%.

In the present study, the measurements were made of wind velocity fluctuations at the Section C of 18.75 m fetch under the several conditions of waves and wind speeds. Usually the number of revolution of blower's fan per minute (rpm) is effectively used for setting up a particular wind speed by adjusting and monitoring it to the order of 0.1 rpm throughout a particular run by means of an electric tachometer. Mainly we used the wind speeds for rpm 100, 200 and 300, respectively. The corresponding mean wind speeds at A section were nearly 2.6, 5.6 and 8.2 m/sec, respectively, and the maximum wind speeds at the measurement station (C section) were roughly 3.1, 6.3 and 9.5 m/sec, respectively, slightly depending on the water surface conditions. The daily differences of the real (measured) maximum wind speed at C section in the same case for the same rpm were less than about 1%.

The measurements were performed under the three different wave conditions; that is, (i) the case of wind waves only, where the wind was blown over the clean still water, (ii) the case of mechanically generated long waves with height of 5.7 cm and period of 1.60 seconds, where the wind waves naturally generated coexisted, and (iii) the case of clear long waves with period of 1.60 seconds, where the suitable amount of soap was applied into water to eliminate wind waves completely and the water surface undulations were nearly sinusoidal. We will call the second condition the case of 'composite long waves' in this report although the term of 'superposed long waves' was used in the previous one. The wave heights of 'clear long waves' used were 1.14, 5.98 and 7.03 cm. The combinations of those winds and waves will be shown later.

A digital data recorder (DATAC-2000, Iwasaki Commun. Instr. Co.) was used for recording the output voltages from the hot-wire systems and the capacitance wave meter. The recorder has three input channels now. After being pre-amplified by the precision DC-amplifiers, if necessary, the input analog data are converted into digital quantities and are recorded on a magnetic tape of 1/2 inch width in a format fitted to the digital computer. The significant figures of the A-D converter are three (-999~+999) and the converting time is less than 150  $\mu$  seconds. The sampling time intervals available are from 1/45 to 1/1440 second for one channel use. The quasi-simultaneous sampling is also possible, where the time lag between the successive channels is 150  $\mu$  seconds. This type of sampling was used for the multi-channel recording throughout the experiments.

## 2.2 Hot-wire Operation

Horizontal (streamwise) and vertical wind velocity fluctuations were measured by means of an X-wire probe and two suits of constant-temperature hot-wire anemometer and linearizer system with a high frequency response, reported by the manufacturer (Nihon Kagaku Kogyo Co.), of 0~50 KHz. The X-wire element consists of two tungsten wire of  $5\mu\phi$  and 1 mm length fixed crossing each other at the angle of  $90^\circ$ . The element is attached to the support of  $8\text{mm}\phi$  in such a way that each hot-wire makes the angle of  $\pm 45^\circ$  to the horizontal direction in a vertical plane parallel to the wind direction as shown in Fig. 2.

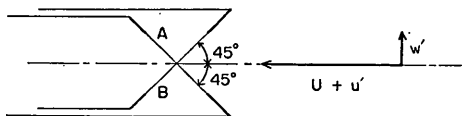


Fig. 2. Orientation of an X-Wire Element

The relation between the output voltage from the anemometer  $E$  and the normal wind speed to a hot-wire  $V$ , for a particular operating condition with constant surrounding temperature, is expressed to a good approximation as

$$E^2 = E_0^2 + a_1 V^{1/2}, \quad (1)$$

where  $E_0$  and  $a_1$  are constants. We feed this voltage  $E$  to the linearizer to obtain the output voltage  $M$  linearly proportional to  $V$  such as

$$M = KV. \quad (2)$$

The coefficients  $K_A$  and  $K_B$  for each wire were obtained from the calibration carried out just before the each day measurements by placing an X-wire and a pitot static tube side by side at the center position ( $z=40\text{ cm}$ ) of the tunnel core portion at C section where the turbulence level was fairly low as will be shown later. For four or five different wind speeds, the mean wind speed  $\bar{V} (=U/\sqrt{2})$  were measured by using the pitot static tube, while the linearizer output voltages were recorded with the digital data recorder described above to obtain the mean voltages  $\bar{M}_A, \bar{M}_B$ . The calibration curves thus obtained, one of which is shown in Fig. 3, were satisfactorily linear in general.

The total voltages,  $X$  and  $Y$ , from the X-wire system described above ( $X$  corresponding to a wire  $A$  in Fig. 2) are related to the wind velocity components by

$$\left. \begin{aligned} X &= \frac{1}{\alpha' \sqrt{2}} \left[ U + (u' + w') + \frac{w'^2}{U} \right. \\ &\quad \left. - \frac{w'^2(u' + w')}{U^2} + \dots \right], \\ Y &= \frac{1}{\beta' \sqrt{2}} \left[ U + (u' - w') + \frac{w'^2}{U} \right. \\ &\quad \left. - \frac{w'^2(u' - w')}{U^2} + \dots \right], \end{aligned} \right\} \quad (3)$$

where  $u', w'$  are the total streamwise and vertical velocity fluctuations, respectively,  $U$  the mean velocity, and  $\alpha', \beta'$  are the constants determined from the calibration ( $\alpha' = K_A$  and  $\beta' = K_B$ ). There-

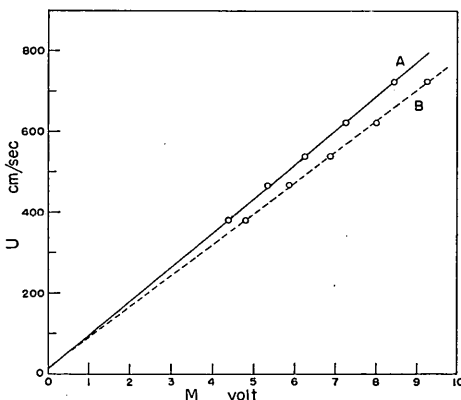


Fig. 3. An example of the X-Wire Calibration Curves

fore the first order fluctuating voltage components,  $X'$  and  $Y'$  are given, respectively, by

$$\left. \begin{aligned} \alpha' X' &= \frac{1}{\sqrt{2}}(u' + w'), \\ \beta' Y' &= \frac{1}{\sqrt{2}}(u' - w'). \end{aligned} \right\} \quad (4)$$

By putting  $\alpha'/\sqrt{2} = \alpha$ ,  $\beta'/\sqrt{2} = \beta$ ,  $u'$  and  $w'$  are obtained as follows:

$$\left. \begin{aligned} u' &= \alpha X' + \beta Y', \\ w' &= \alpha X' - \beta Y'. \end{aligned} \right\} \quad (5)$$

Usually  $u'$  and  $w'$  may be obtained by the analog method from (5); if the sensitivities of two linearizers are adjusted to produce the same conversion coefficients such as  $\alpha = \beta$ , then the adding and subtracting circuits give  $u'$  and  $w'$ , respectively. In the present experiments, however, the instantaneous total voltages,  $X$  and  $Y$ , were recorded in the digital form at the small time interval  $\Delta t$  by means of the digital data recorder, and  $u'$  and  $w'$  were evaluated at the interval  $\Delta t$  from the relation (5) by using a computer. This digital method enabled us not only to save time in setting the instruments but also to apply the precise conversion coefficients  $\alpha$  and  $\beta$ .

### 2.3 Velocity Measurement

At the wind field over water waves, the total velocity fluctuations  $u'$ ,  $w'$  are resolved as follows:

$$\left. \begin{aligned} u' &= \bar{u} + u_i', \\ w' &= \bar{w} + w_i', \end{aligned} \right\} \quad (6)$$

where  $\bar{u}$ ,  $\bar{w}$  are the wave induced periodic components and  $u_i'$ ,  $w_i'$  the turbulent components. Since the measurements were made in the middle vertical plane of the tunnel width, the air flow there may be assumed as two-dimensional. In the present study we measured the wave induced components as the phase-average values with respect to time, not the mean values with respect to the transverse coordinate (cf. Phillips, 1966, p. 90). That is, for the case of clear long waves in our experiments,

$$\left. \begin{aligned} \bar{u}(t_\epsilon) &= \langle u' \rangle_\epsilon = \frac{1}{N} \sum_{j=1}^N u'(t_\epsilon + jT_p), \\ \bar{w}(t_\epsilon) &= \langle w' \rangle_\epsilon = \frac{1}{N} \sum_{j=1}^N w'(t_\epsilon + jT_p), \end{aligned} \right\} \quad (7)$$

where  $t_\epsilon$  denotes a time corresponding to a particular wave phase,  $\langle \rangle_\epsilon$  the time average with respect to a particular wave phase  $\epsilon$  and  $T_p$  the wave period. If  $\langle \rangle$  is simply used in the following, a particular phase is related implicitly in the case of clear long waves. For the case of wind waves only, however, such averaging was not possible and only the total power of the wave induced components was roughly estimated.

In the present study we made use of the cross spectral analysis with respect to  $u'$  and  $w'$  in the digital way extensively, because we thought this was the only means to detect the wave-induced velocity components, especially the wave-induced Reynolds stresses in the cases of wind waves only ( $u'$ - and  $w'$ -spectra were obtained as by-products). The computation procedures used were the same as those in the previous report (Kato & Sano, 1969) and of general type. We usually took  $\Delta t=1/30$  second in these computations to obtain the frequency resolution  $\Delta f=0.1$  Hz with a maximum lag of 150, yielding the holding frequency  $f_N=15$  Hz. There was naturally the same problems of aliasing as discussed in some detail for the computations of auto-spectra in the previous report (Kato & Sano (1969), see also Blackman & Tukey (1958)). To eliminate the aliased power, we occasionally used

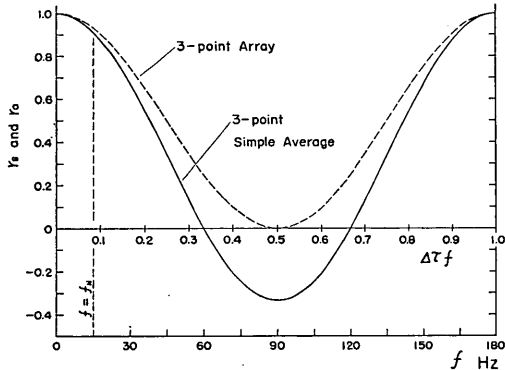


Fig. 4. Frequency Response of the 3-Point Sliding Mean and Array

a simple numerical filter (3-point sliding mean) as follows:

$$y_i = \frac{1}{3}(x_{i-1} + x_i + x_{i+1}), \quad (8)$$

where  $x_i$  is input data and  $y_i$  output of  $i$ -th. If the original sampling time interval is  $\Delta\tau$ , the attenuation factor  $r_s$  at frequency  $f$  is given by

$$r_s(f) = \frac{1}{3}(1 + 2 \cos 2\pi f \Delta\tau). \quad (9)$$

The relation (9) is shown in Fig. 4, together with that for a 3-point array:

$$\left. \begin{aligned} y_i &= 0.25x_{i-1} + 0.5x_i + 0.25x_{i+1}, \\ r_a(f) &= \frac{1}{2}(1 + \cos 2\pi f \Delta\tau). \end{aligned} \right\} \quad (10)$$

It is found from Fig. 4 (where  $\Delta t=6\Delta\tau$ ) that the filter not only eliminates the aliased power but also cut down the required components at the frequencies lower than  $f_N$ . The results obtained by this filter (8) with  $\Delta t=1/180$  second are distinguished by  $M$  attached to the run-number in the later figures, like Y3312-M.

The simpler computation procedures were also used to obtain just  $\overline{u'w'}$ ,  $\overline{u'^2}$  and  $\overline{w'^2}$  for all cases, and the following relation was also useful:

$$\overline{u'w'} = \alpha^2[\overline{X^2} - (\overline{X})^2] - \beta^2[\overline{Y^2} - (\overline{Y})^2], \quad (11)$$

where  $\overline{\quad}$  denotes overall time average. In the case of clear long waves, we measured  $\bar{u}$  ( $=\langle u' \rangle$ ),  $\bar{w}$ ,  $\langle u'^2 \rangle$ ,  $\langle w'^2 \rangle$  and  $\langle u'w' \rangle$  by applying the procedure shown in (7), from which further  $\bar{u}\bar{w}$ ,  $\langle u_i'^2 \rangle$ ,  $\langle w_i'^2 \rangle$  and  $\langle u_i'w_i' \rangle$  were estimated by using the following relations:

$$\left. \begin{aligned} \langle u_i'^2 \rangle &= \langle u'^2 \rangle - (\bar{u})^2, \\ \langle w_i'^2 \rangle &= \langle w'^2 \rangle - (\bar{w})^2, \\ \langle u_i'w_i' \rangle &= \langle u'w' \rangle - \bar{u} \cdot \bar{w}. \end{aligned} \right\} \quad (12)$$

In these cases we took special care for the period of wave generator to be exactly 1.60 seconds; practically the deviation of total time corresponding to 300 wave periods was reduced to less than 0.2 second by using a stopwatch. For the cases of the wave height  $H=6.0$  cm, we recorded the water surface undulations at C section simultaneously with the wind velocity data by means of a capacitance wave gauge placed near one side wall of the wind-wave tunnel. These wave data indicated that there existed a small shift of the wave period. Usually the sampling time interval was  $\Delta t=1/30$  and the total data number for a single number for a single computation  $N=6000$ . The position of wave crest, which should appear at every 48 data, shifted in the worst case by 6 or 7 after 124 waves ( $=5952$  data). It was found later that the mean values simply evaluated with respect to every 48 data gave fairly good results, even if the phase shift of this much existed. However, in the final computations of the relations such as (12) for the cases of  $H=6.0$  cm we made corrections so that the computed and real phase positions might not drift apart by more than one data. In the cases of  $H=1.14$  cm such a correction was not made because a slight level of the noise voltages from the capacitance bridge somewhat obscured the real crest positions.

### 3. Results and Discussion

#### 3.1 The Cases of Clear Long Waves

In these cases the waves with a single period of 1.60 seconds were generated by a wave generator, the wind waves being suppressed completely by applying soap into water. The wave heights used in the experiment were mainly  $H=6.0$  cm and  $H=1.14$  cm, and supplementally  $H=7.0$  cm. The wind speed used were those established at rpm 200 and rpm 100. The corresponding mean wind velocity profiles in the cases of  $H=6.0$  cm and  $H=1.14$  cm are shown in Figs. A-1 and A-2 in the Appendix. Note that a critical layer, where  $U=c$  ( $\approx 192$  cm/sec), appeared at the height of  $z=3$  cm in the case of  $H=1.14$  & rpm 100, while in the other cases all measurements were made far above the critical layers. All experiments in these cases were conducted at the room temperature of  $15.0\sim 19.0^\circ\text{C}$  and that of water  $14.0\sim 15.5^\circ\text{C}$  in November, and it is likely that the hot-wire results were not effected seriously by the seasonal difference, whatever the cause may be, which will be discussed later.

The  $u'$ - and  $w'$ -spectra at a few heights in each case are shown in Figs. A-3 to A-6 to observe the relative magnitudes of wave-induced components which appear near the frequency of clear long waves,  $f_L=0.625$  Hz. It is found from Figs. A-5 and A-6 that in the cases of  $H=1.14$  cm strong wave-induced powers exist in  $u'$ -spectra rather than  $w'$  at the lowest height ( $z=1.2$  or  $1.5$  cm) above the wave crests and that they are reduced rapidly with increasing height. On the other hand, the dominant powers induced by waves appear in  $w'$  rather than  $u'$  in the case of  $H=6.0$  & rpm 200 and in both  $u'$  and  $w'$  in the case of  $H=6.0$  & rpm 100 at the lowest height ( $z=4.0$  cm) as seen from Figs. A-3-(a) and A-4-(a), respectively. It is further seen that these wave-induced powers do not decrease remarkably with height, which was already pointed out in the previous report (Kato & Sano, 1969).

The variations of co- and quad-spectra with height in each case mentioned above are shown in Figs. 5~8, and the corresponding coherences  $\hat{\rho}^2$  are shown

in Figs. A-7~A-10 in Appendix. The whole results at  $z=4$  cm in the case of  $H=7.0$  cm & rpm 200 are shown in Fig. 9. The digital computations were performed with  $\Delta t=1/30$  second,  $N=6000$  and maximum lag  $h=150$ . The spectral densities for  $f>7$  Hz are omitted in these figures because no significant power existed there. The remarkable variations of the spectral density around  $f\approx 0.6$  Hz are considered to correspond to the wave-induced components; the projection (or depression) of co-spectral density there indicates the existence of the positive (or negative) wave-induced Reynolds stress. Since  $-\overline{u'w'}$  is related to the co-spectral density by

$$-\overline{u'w'} = -\int_0^{\infty} C_{u'w'}(f)df, \quad (13)$$

it is expected that the area, which is the projecting (or depressing) part and a line connecting the both side showing no wave-induced power, is nearly equal to to the value of  $-\overline{u\bar{w}}$  (see Fig. 10).

On the other hand, the phase averaging described earlier (p. 11) yielded  $\bar{u}$  and  $\bar{w}$ , from which  $-\overline{u\bar{w}}$  were evaluated directly.  $-\overline{u\bar{w}}$  thus obtained are shown

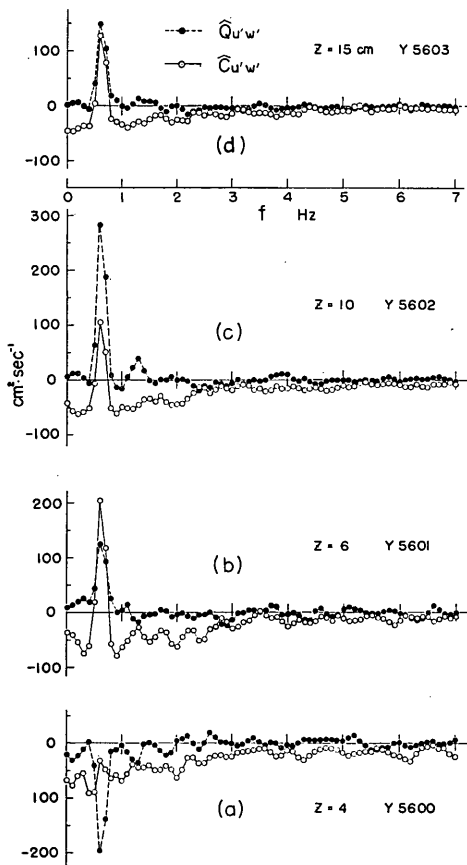


Fig. 5. Co- and Quad-Spectra in the cases of clear long waves; rpm 200,  $H=6.0$  cm

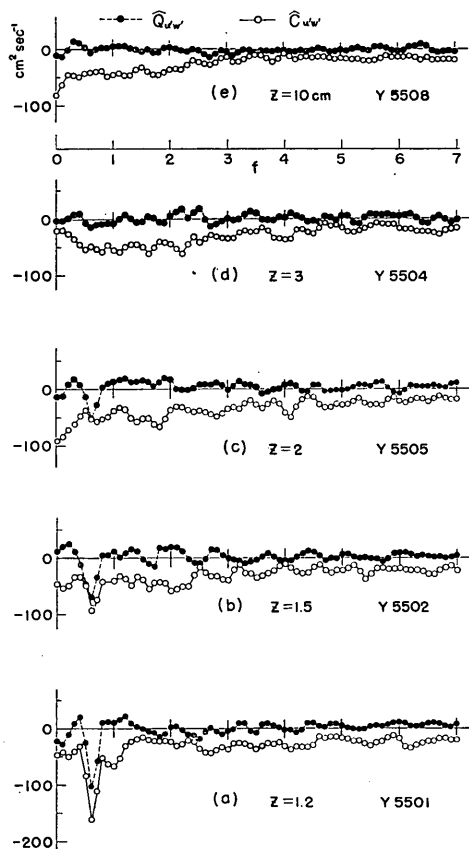


Fig. 6. Co- and Quad-Spectra in the cases of clear long waves; rpm 200,  $H=1.14$  cm

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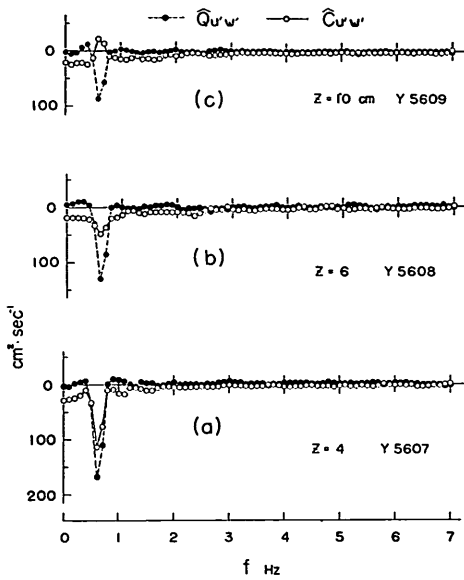


Fig. 7. Co- and Quad-Spectra in the cases of clear long waves; rpm 100,  $H=6.0$  cm

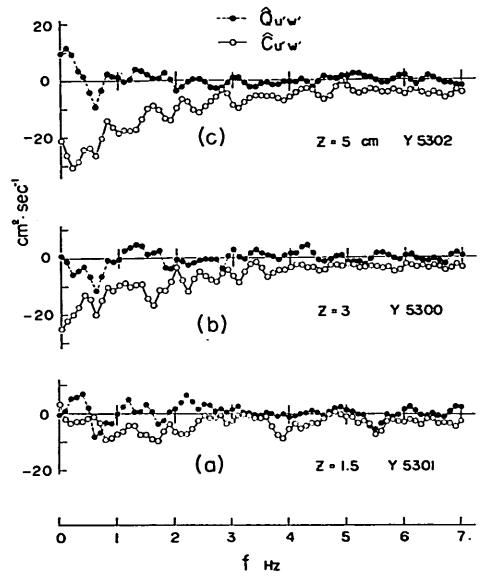


Fig. 8. Co- and Quad-Spectra in the cases of clear long waves; rpm 100,  $H=1.14$  cm

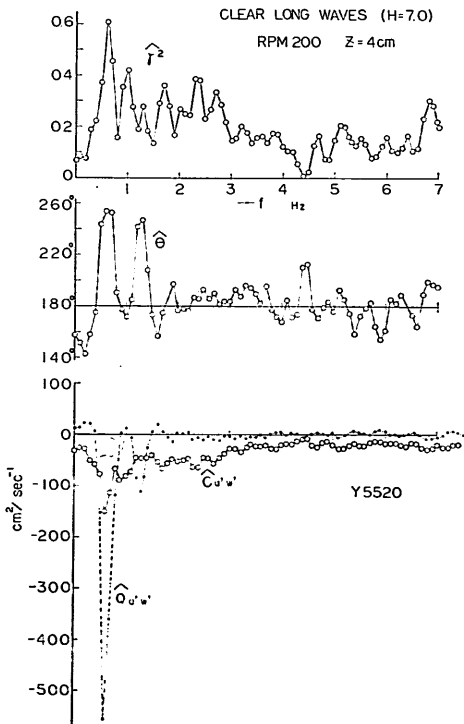


Fig. 9. Cross Spectrum in the case of clear long waves; rpm 200,  $H=7.0$  cm,  $z=4$  cm

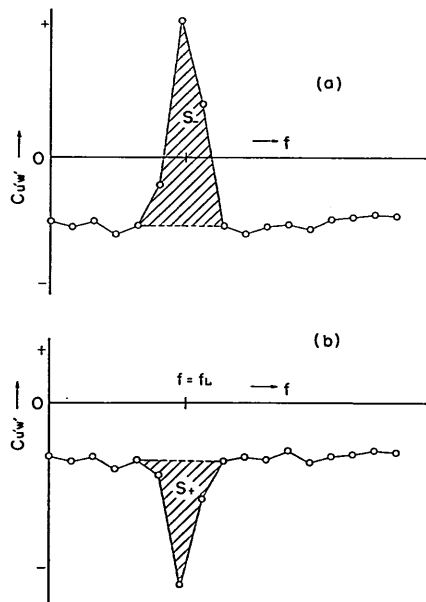


Fig. 10. Graphical Estimation of the Wave-Induced Reynolds stresses: Areas  $S_-$  in (a) and  $S_+$  in (b) are nearly equal to the negative and positive stresses, respectively

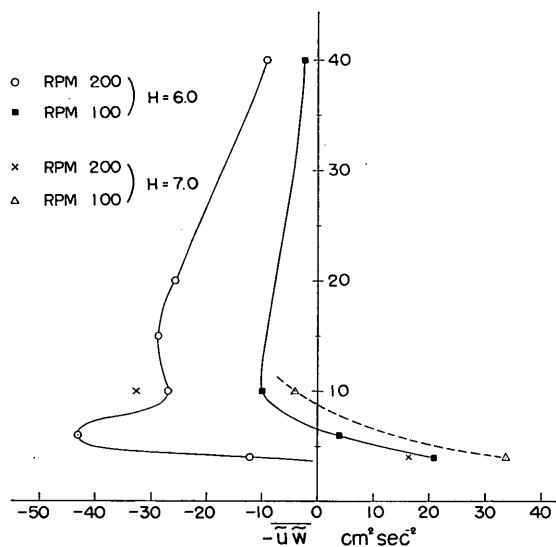


Fig. 11. Wave-Induced Reynolds Stresses in the cases of clear long waves; rpm 200 & 100,  $H=6.0$  &  $7.0$  cm

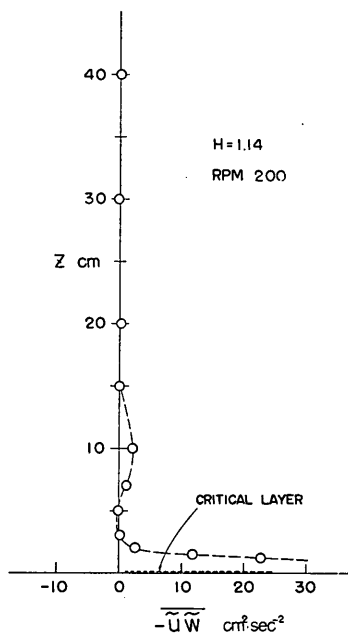


Fig. 12. Wave-Induced Reynolds Stresses in the case of clear long waves; rpm 200,  $H=1.14$  cm

in Figs. 11~13. The values of  $-\overline{u\tilde{w}}$  estimated roughly from the variations of co-spectra in the way shown in Fig. 10 coincided with those from phase averaging very well (within 3%) for the cases of Figs. 6-a and 7-a and reasonably well (10~20%) for other cases with detectable wave-induced powers.

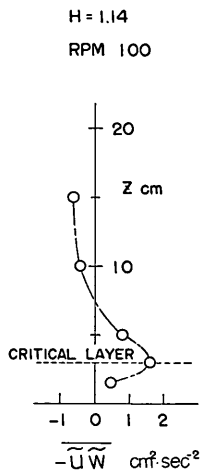


Fig. 13. Wave-Induced Reynolds Stresses in the case of clear long waves; rpm 100,  $H=1.14$  cm

According to the inviscid theory by Miles (1957), the wave-induced Reynolds stress exists only below a critical layer;  $\tilde{u}$  and  $\tilde{w}$  at the height above the critical layer are just  $90^\circ$  out of phase, yielding  $\overline{\tilde{u}\tilde{w}}=0$  there. Recently Hamada (1968) introduced the instability Reynolds stresses other from Miles theory which might exist near the water surface but above the critical layer, if the separation or sliding of air flow took place at the water surface. Hamada expected this would be an effective cause of the deviation of wind speed near the water surface in a wind-wave tunnel from the so-called log-law.

The wave-induced Reynolds stresses measured in the cases of 'clear long waves' appeared in a fairly complicated way. The results in the case of  $H=1.14$  & rpm 100 may be less reliable because of their small values, but in other cases the wave-induced Reynolds stresses definitely existed above the critical layer. In the case of  $H=1.14$  & rpm 200 (shown in Fig. 12), positive wave-



induced Reynolds stress appeared at the lowermost measurement height ( $z=1.2$  cm) above the wave crests and they decreased very rapidly with  $z$ , becoming substantially to zero at  $z=3$ cm, but no negative wave-induced Reynolds stress appeared. In the cases of  $H=6.0$  or  $7.0$ cm (Fig. 11) the wave-induced Reynolds stresses had positive values or a strong tendency toward positive side at the lowermost measurement height, but they became definitely negative at a little larger height. These negative stresses remained until the height of  $z=40$  cm although the absolute values decreased very slowly with  $z$ .

The appearance of negative wave-induced Reynolds stress in the cases with larger waves was rather unexpected result, and the authors suspect that this implies a peculiarity of the experiment conducted in a wind-wave tunnel fundamentally different from the phenomena in the open field conditions. The similar results were also obtained by Kendall (1970) in the measurement of  $-\overline{u'w'}$  above the mechanically generated wave train in a sheet of smooth neoprene rubber. On the other hand, Shemdin & Lai (1970) reported nearly contrary results with respect to the signs of the stress, which will be discussed later.

For the cases of  $H=6.0$  cm (both rpm 200 and rpm 100), we also made an attempt to estimate the double correlation terms  $\langle u_i'^2 \rangle$ ,  $\langle w_i'^2 \rangle$ , and  $\langle u_i'w_i' \rangle$ , by the method described earlier (p. 12), which were originally introduced by Phillips (1966, p. 97) to appear into the vorticity equation governing the wave-induced turbulent air motion. Those quantities together with  $\bar{u}$ ,  $\bar{w}$  and  $\bar{u}\bar{w}$  are shown in Figs. 14~18. A direction entitled 'down wind direction' shown in each figure indicates which of the two phase positions taken on the abscissa was situated leeward. There exist considerable scatter especially in  $-\langle u_i'w_i' \rangle$ , which is probably attributed to the lack of the total wave number for averaging. The vertical distributions of the turbulent Reynolds stress,  $-\overline{u_i'w_i'}$ , in the cases of  $H=1.14$  cm, where the measurement could be made relatively close to the mean water surface, are shown in Fig. 19. For the case of rpm 200 the distribution is similar to those measured in the boundary layer on the fixed wall (e.g. Hinze 1959, p. 491), and the stress tends to be constant at the heights lower than about 2.5cm. On the other hand, for the case of rpm 100  $-\overline{u_i'w_i'}$  again decreases to

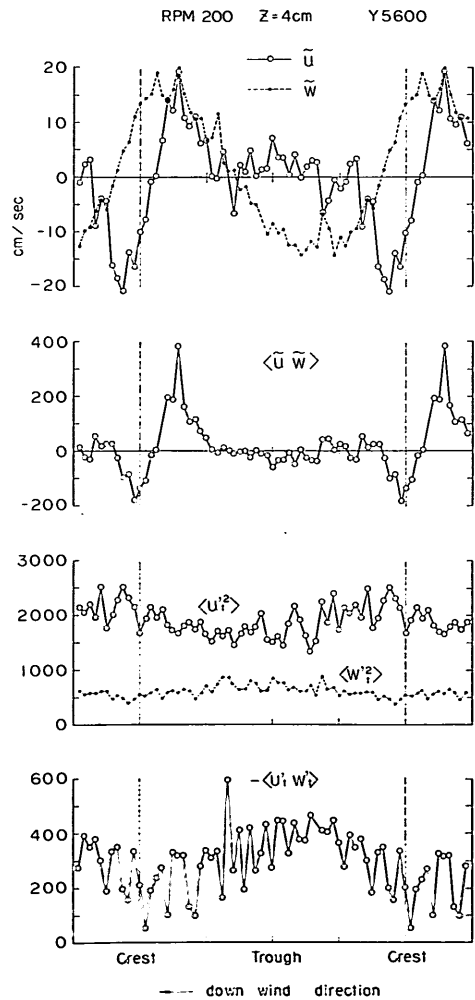


Fig. 14.  $\bar{u}$ ,  $\bar{w}$ ,  $\langle \bar{u}\bar{w} \rangle$ ,  $\langle u_i'^2 \rangle$ ,  $\langle w_i'^2 \rangle$  and  $-\langle u_i'w_i' \rangle$  in the case of clear long waves; rpm 200,  $H=6.0$  cm,  $z=4$  cm

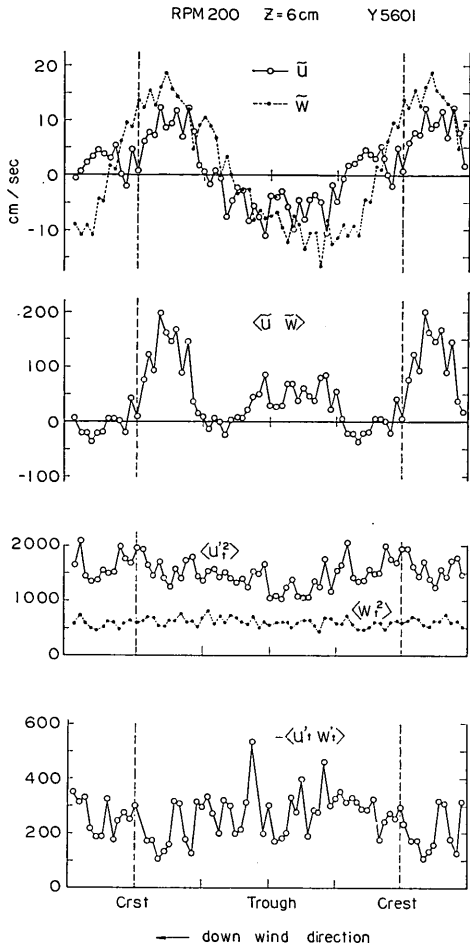


Fig. 15.  $\bar{u}$ ,  $\bar{w}$ ,  $\langle \bar{u} \bar{w} \rangle$ ,  $\langle u_i'^2 \rangle$ ,  $\langle w_i'^2 \rangle$  and  $-\langle u_i' w_i' \rangle$  in the case of clear long waves; rpm 200,  $H=6.0$  cm,  $z=6$  cm

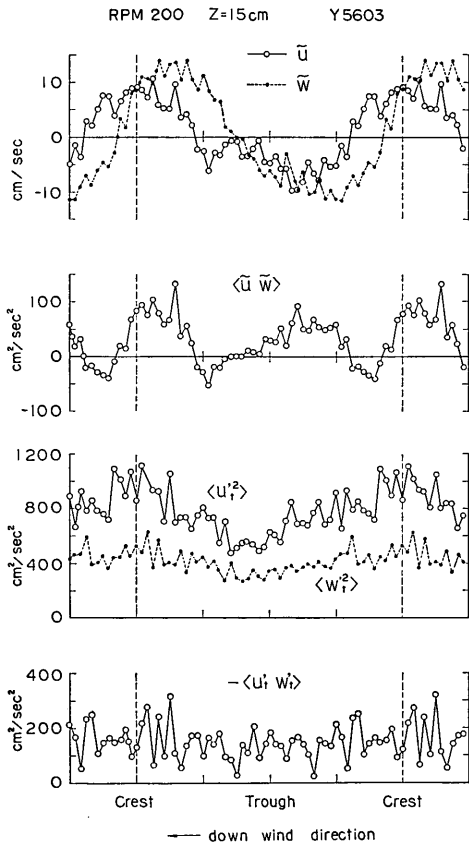


Fig. 16.  $\bar{u}$ ,  $\bar{w}$ ,  $\langle \bar{u} \bar{w} \rangle$ ,  $\langle u_i'^2 \rangle$ ,  $\langle w_i'^2 \rangle$  and  $-\langle u_i' w_i' \rangle$  in the case of clear long waves, rpm 200,  $H=6.0$  cm,  $z=15$  cm

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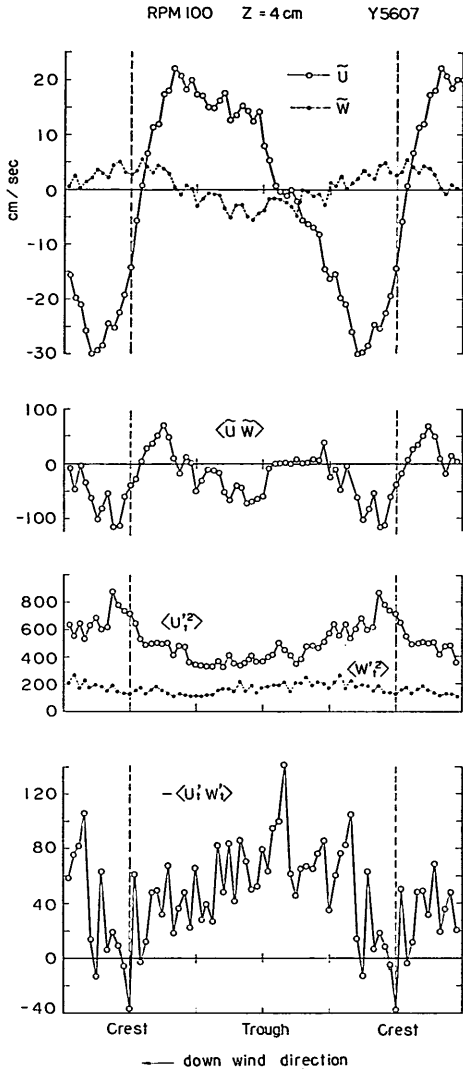


Fig. 17.  $\tilde{u}$ ,  $\tilde{w}$ ,  $\langle \tilde{u}\tilde{w} \rangle$ ,  $\langle u_i'^2 \rangle$ ,  $\langle w_i'^2 \rangle$  and  $-\langle u_i'w_i' \rangle$  in the case of clear long waves; rpm 100,  $H=6.0$  cm,  $z=4$  cm

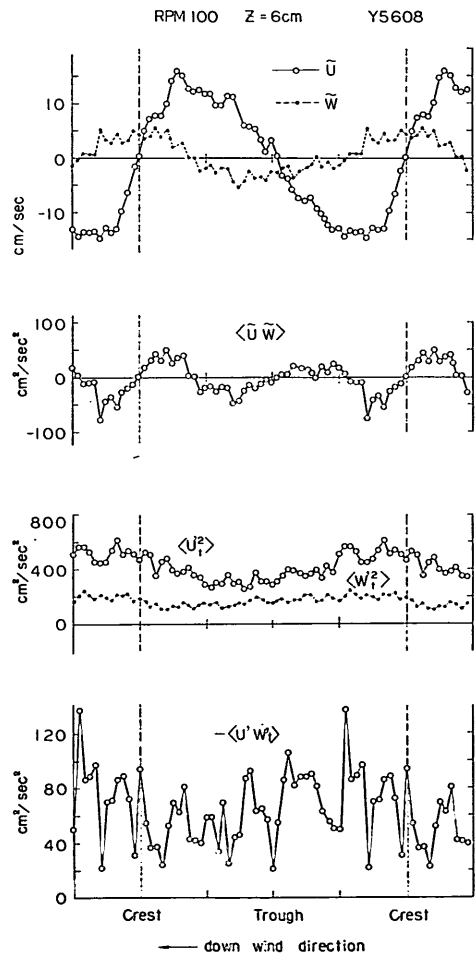


Fig. 18.  $\tilde{u}$ ,  $\tilde{w}$ ,  $\langle \tilde{u}\tilde{w} \rangle$ ,  $\langle u_i'^2 \rangle$ ,  $\langle w_i'^2 \rangle$  and  $-\langle u_i'w_i' \rangle$  in the case of clear long waves; rpm 100,  $H=6.0$  cm,  $z=6$  cm

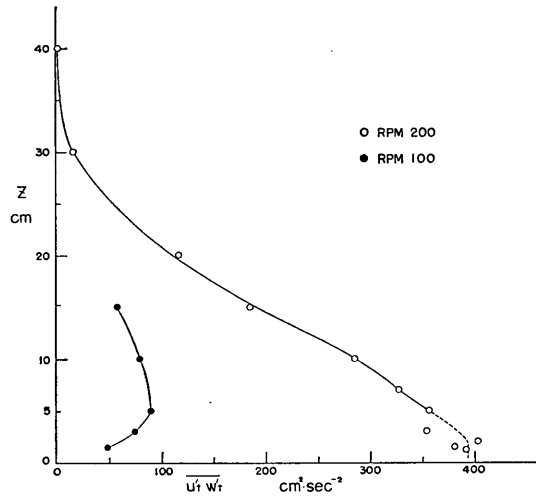


Fig. 19. Distributions of Turbulent Reynolds Stress in the cases of clear long waves; rpm 200 & 100,  $H=1.14$  cm

ward the water surface at  $z < 5$  cm.

### 3.2 The Cases of Wind Waves Only

In this case the measurements at the wind speeds for rpm 300 were made twice and the measurements for rpm 200 were repeated several times throughout a year, because not only quantitative but also qualitative inconsistency was found in the values of total Reynolds stress obtained. As to the waves at the measurement station (C section) in these cases, the peak frequency  $f_p$  of the wave spectra and  $H_{1/3}$  obtained from the total power of the spectra were  $f_p \doteq 2.5$  Hz,  $H_{1/3} = 2.5$  cm for rpm 200 and  $f_p \doteq 2.0$  Hz,  $H_{1/3} = 4.3$  cm for rpm 300.

The variations of the obtained  $-\overline{u'w'}$  with height from the mean water surface are shown in Figs. 20 and 21. Fig. 20 for rpm 200 includes all the results in the measurements made at different seasons together with the experimental dates and room temperatures. At a glance, it is observed from the both figures that much scatter exists in the values of  $-\overline{u'w'}$  near the water surface. Looking at carefully, it is found further that there is no doubt a tendency that the values of  $-\overline{u'w'}$  near the water surface obtained at the season of high temperature (summer) are larger than those at low temperature (winter). Moreover, in the measurements at the time of high temperature the values of  $-\overline{u'w'}$  increased monotonously with decreasing height, while at the lower temperature the maximum values turned up at a certain height  $z_m$  ( $z_m \doteq 5 \sim 6$  cm for rpm 200 and  $z_m \doteq 8 \sim 10$  cm for rpm 300) above the wave crests and below the height  $-\overline{u'w'}$  decreased toward the water surface. It is seen from Fig. 20 that the lower the temperature was, the greater reduction in  $-\overline{u'w'}$  took place at the lowermost measurement height. The variations of  $\overline{u'^2}$  and  $\overline{w'^2}$ , which were measured simultaneously with  $-\overline{u'w'}$  are shown in Figs. A-11 and A-12 in Appendix. Although there is considerable scatter in the values of  $\overline{u'^2}$  and  $\overline{w'^2}$  also, the seasonal trend can not be found so clearly as in  $-\overline{u'w'}$ . So far we have not been able to ascertain whether such a difference was caused simply by the instrumental errors or there is any trend in the natural wind properties.

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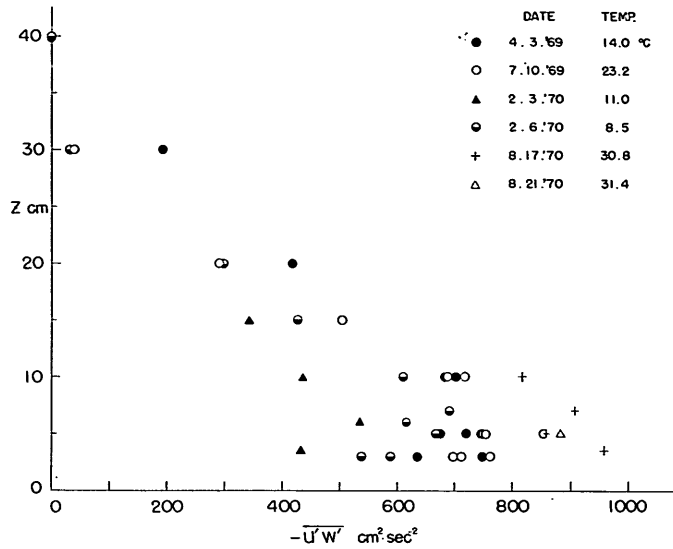


Fig. 20. Distributions of  $-\overline{u'w'}$  in the cases of wind waves only; rpm 200

One thing we can suspect is that the difference of the vertical temperature distribution over the water surface may have resulted in such an inconsistency to some extent. We carried out the hot-wire calibration at the height of  $z=40$  cm above the water surface as described previously, and the temperature of flowing air near the water surface is usually somewhat lower than that at the calibration height. This could, more or less, change the hot-wire calibration constants near the water surface. The influence of this change appears directly in the total output voltage, that is in the measurement of mean velocity. Since, however, this effect is considered to be much less influential in the measurement

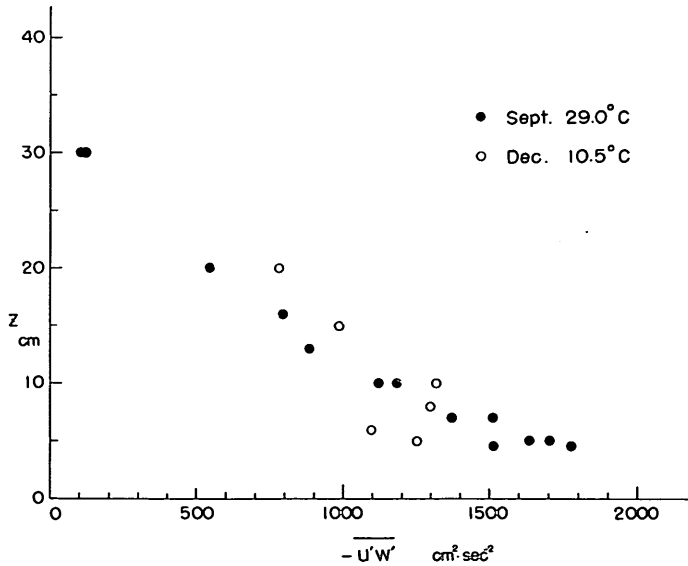


Fig. 21. Distributions of  $-\overline{u'w'}$  in the cases of wind waves only; rpm 300

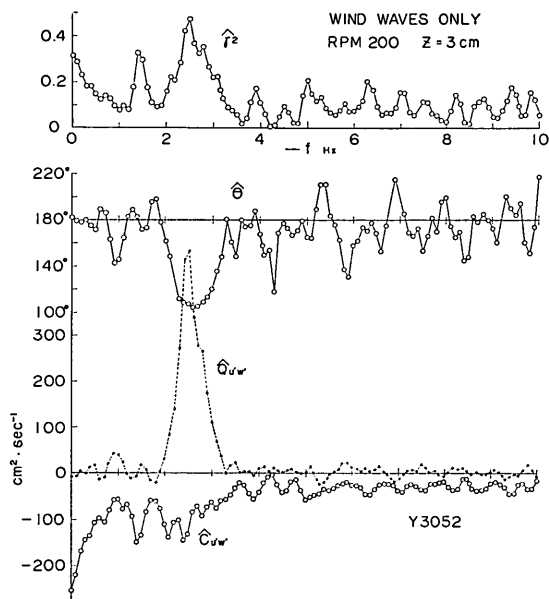


Fig. 22.  $u'w'$  Cross-Spectrum in the case of wind waves only; rpm 200,  $z=3$  cm

of the fluctuating components, we have made no correction for this effect. The temperature difference  $\Delta T$  between air (in the laboratory) and water is usually in the range of  $4^\circ$  to  $5^\circ\text{C}$ , but it increases up to about  $6^\circ\text{C}$  in summer and decreases down to about  $2^\circ\text{C}$  or less in winter (in autumn  $\Delta T$  occasionally becomes to nearly zero). When  $\Delta T=6^\circ\text{C}$  in summer, the temperature difference of flowing air  $\Delta T_1$  between near the water surface and at  $z=40$  cm could mount up to about  $3^\circ\text{C}$  for rpm 200 (cf. Kato & Takemura, 1966). When  $\Delta T=2^\circ\text{C}$  or less in winter,  $\Delta T_1$  must be about  $1^\circ\text{C}$  or less. Such a degree of difference of  $\Delta T_1$ , however, can hardly be considered to have produced the seasonal difference of the

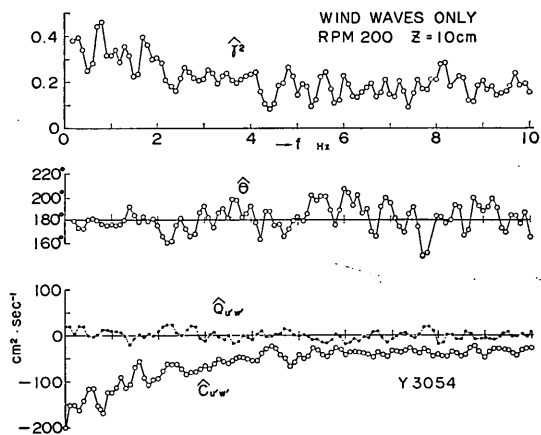


Fig. 23.  $u'w'$  Cross-Spectrum in the case of wind waves only; rpm 200,  $z=10$  cm

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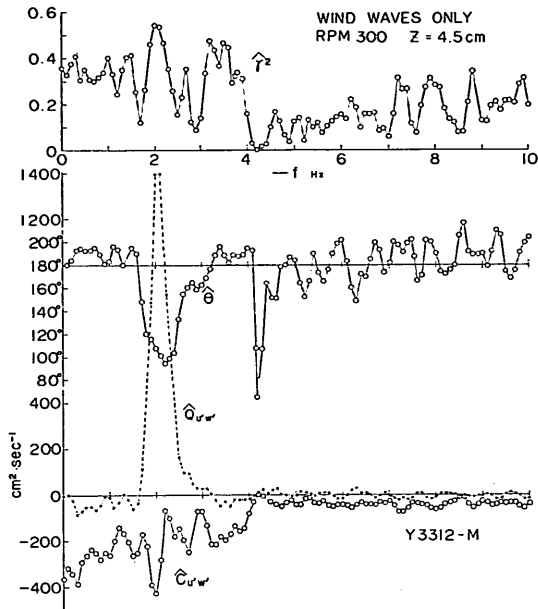


Fig. 24.  $u'w'$  Cross-Spectrum in the case of wind waves only; rpm 300,  $z=4.5$  cm

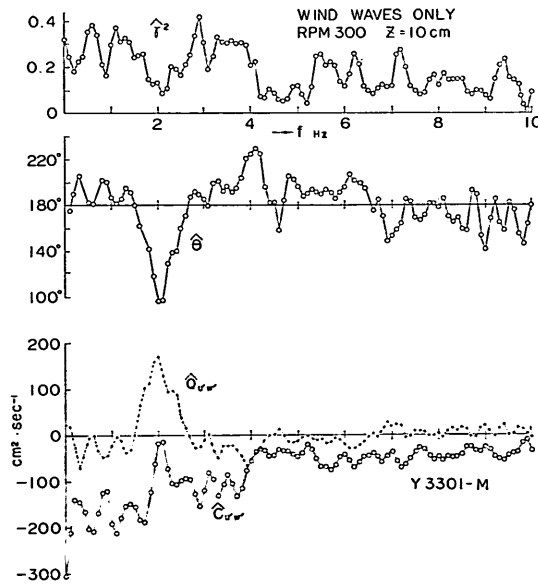


Fig. 25.  $u'w'$  Cross-Spectrum in the case of wind waves only; rpm 300,  $z=10$  cm

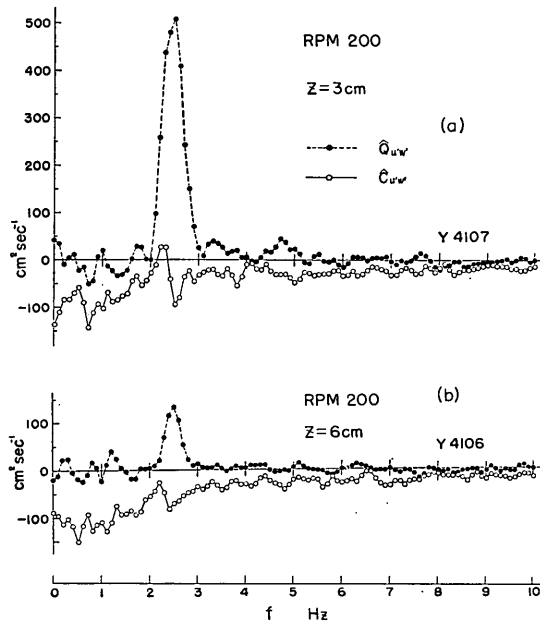


Fig. 26. Co- and Quad-Spectra in the cases of wind waves only; rpm 200; (a)  $z=3 \text{ cm}$ , (b)  $z=6 \text{ cm}$

results so much as shown in Figs. 20 and 21.

As mentioned above the flowing air over the water surface was in more stable condition in summer than in winter. In a stably stratified shear flow the Reynolds stress is usually smaller than in a neutral flow. The seasonal difference of  $-\overline{u'w'}$  found in the present experiment contradicts to the fact also.

Concerning the results of the cross spectra between  $u'$  and  $w'$ , the effect of the different temperature (or season) was hardly observed except a weak tendency found in the variations of cospectra near the frequency  $f_p$ . Some of the whole cross spectral results, namely co- and quad-spectra  $\hat{C}_{u'w'}$  and  $\hat{Q}_{u'w'}$ , phase

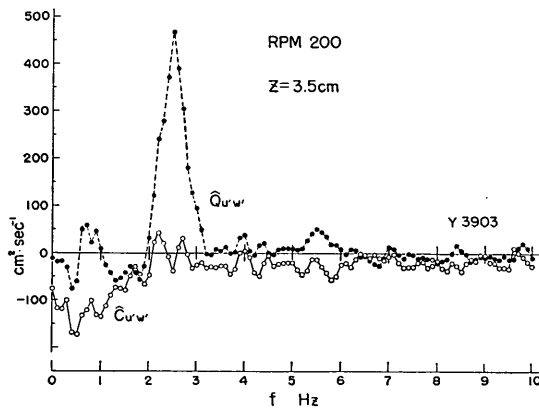


Fig. 27. Co- and Quad-Spectrum in the case of wind waves only; rpm 200,  $z=3.5 \text{ cm}$



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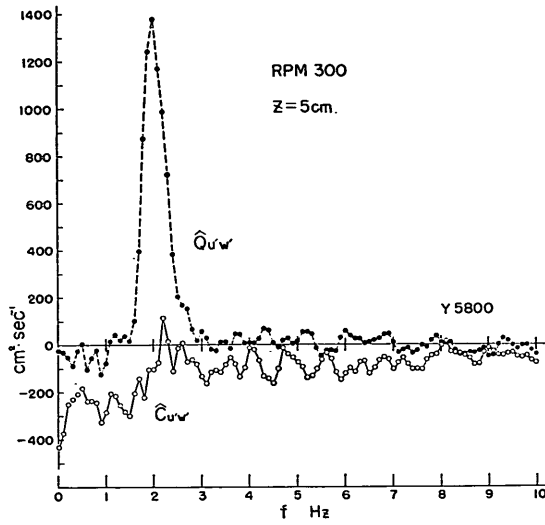


Fig. 28. Co- and Quad-Spectrum in the case of wind waves only; rpm 300,  $z=5$  cm

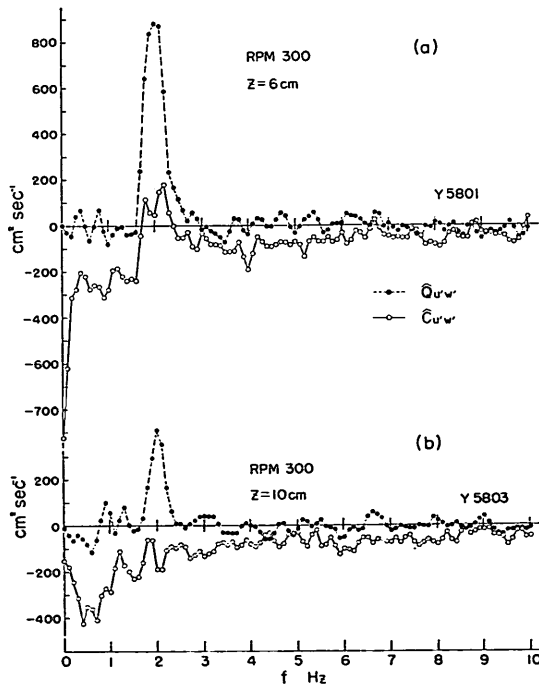


Fig. 29. Co- and Quad-Spectra in the cases of wind waves only; rpm 300; (a)  $z=6$  cm, (b)  $z=10$  cm

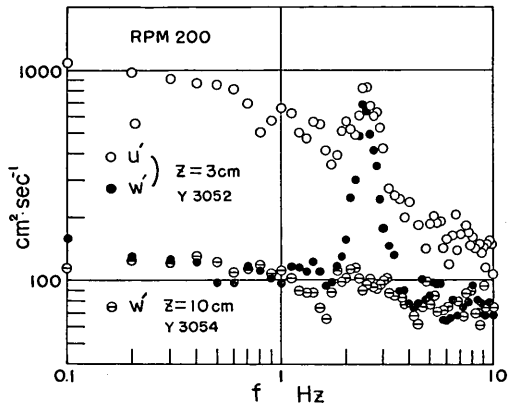


Fig. 30.  $u'$ - and  $w'$ -Spectra in the cases of wind waves only; rpm 200

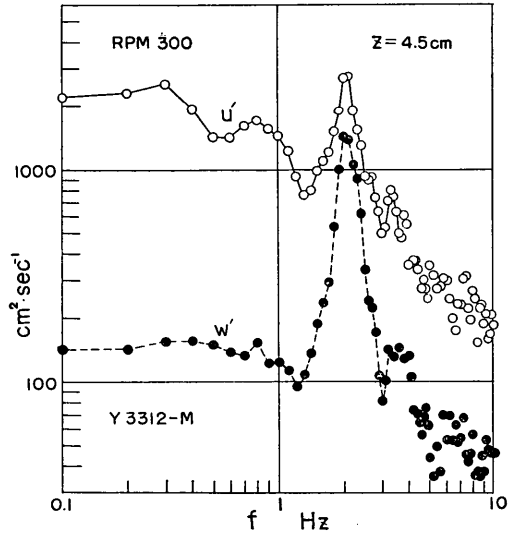


Fig. 31.  $u'$ - and  $w'$ -Spectrum in the case of wind waves only; rpm 300,  $z=4.5\text{cm}$

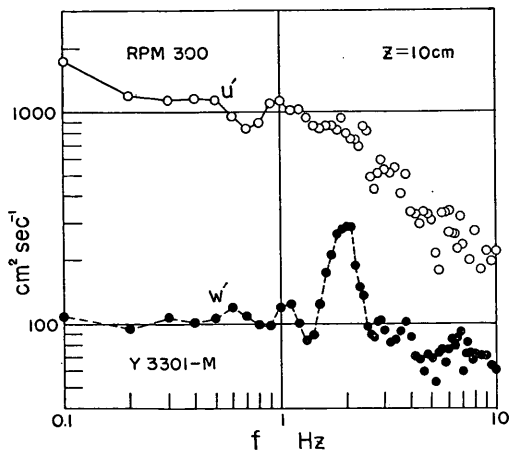


Fig. 32.  $u'$ - and  $w'$ -Spectrum in the case of wind waves only; rpm 300,  $z=10\text{cm}$

angles  $\hat{\theta}$  (which represents the phase lags of the  $w'$ -record relative to the  $u'$ -record) and coherences  $\hat{\gamma}^2$ , are shown in Figs. 22 and 23 for rpm 200 (at Temp.  $\doteq 23^\circ\text{C}$  in July) and in Figs. 24 and 25 for rpm 300 (at Temp.  $\doteq 29^\circ\text{C}$  in Sept.), which were obtained at the seasons of relatively high temperature as indicated in the parentheses above. The results of co- and quad-spectra, which were obtained at the winter season, are shown in Fig. 26-a, b (at Temp.  $= 8.5^\circ\text{C}$ ) and Fig. 27 (Temp.  $\doteq 11.5^\circ\text{C}$ ) for rpm 200 and in Figs. 28 and 29-a, b for rpm 300 (Temp.  $\doteq 10.5^\circ\text{C}$ ). Examples of  $u'$ - and  $w'$ -spectra are shown in Figs. 30~32.

As seen from Figs. 30 and 31, there exist considerable wave-induced velocity fluctuations, which appear near the frequency  $f_p$  in the  $u'$ - and  $w'$ -spectra, at the lowermost measurement heights above the wave crests and the bulk of  $w'$ -power there consists of the wave-induced component. Figs. 22 and 24 indicate that these wave-induced  $u'$  and  $w'$  components are nearly  $90^\circ$  out of phase ( $u'$  component is leading) and that they do not make much contribution to the cospectra if any, although a strong peak appear near the frequency  $f_p$  in the quad-spectra. However, most of the cospectra at the small heights obtained in winter show signs of negative wave-induced Reynolds stress in the same way as illustrated in Fig. 10. For the case of rpm 300, shown in Figs. 28 and 29-a, the magnitudes of the negative stresses estimated by the graphical method of Fig. 10 are nearly as large as the magnitudes of the reduction in  $-\overline{u'w'}$  near the water surface (see also Fig. 21). For the cases of rpm 200, the magnitudes of the negative stresses estimated in the same way seem to be a little smaller than those of the reduction in  $-\overline{u'w'}$  near the water surface, and the reduction may be distributed in a wider frequency range. On the other hand, some of the  $u'w'$ -cospectra obtained at the summer season even show, in fact, slight indications of the positive wave-induced Reynolds stress. Such a typical example is presented in Fig. A-13 in Appendix, but note that this is the same experimental case as that shown in Figs. 24 and 25.

At the height of  $z=10$  cm, practically no wave-induced power can be found for rpm 200 as seen both from the  $w'$ -spectrum in Fig. 30 and from the co- and quad-spectrum in Fig. 23, and the phase lags between  $u'$  and  $w'$  at that height are nearly  $180^\circ$  at all frequencies, simply showing a feature of the turbulent shear flow. For the case of rpm 300 a small amount of wave-induced power still remains at that height, especially in  $w'$  as seen from Fig. 32.

The mean velocity profiles in neutral air over the water waves are also widely believed to be represented by

$$U(z) = \frac{u_*}{\kappa} \log_e \frac{z}{z_0}, \quad (14)$$

where  $U(z)$ : mean wind velocity,  $z$ : height above the mean water surface,  $u_*$ : friction velocity ( $=\sqrt{\tau_0/\rho_a}$ ),  $\tau_0$ : wind shear stress at the water surface,  $\rho_a$ : density of air,  $z_0$ : a constant called 'roughness length', and  $\kappa$ : a constant called 'Karman's constant' usually taken to be 0.4.

The relation (14), which is called log-law, has been well established for the boundary layer over the fixed rough surface; the wind profiles over the fixed surface usually exhibit a good logarithmic portion, which is certainly represented by (14) through the real  $\tau_0$ , at the height from the surface to 10~20% of the boundary layer (e.g. Clauser, 1956).

On the other hand, this relation has not been confirmed critically for the wind profiles over the water waves and there are still ambiguous problems in applying

this 'log-law' to those wind profiles, especially in a wind-wave tunnel. It is largely because there is no means to measure  $\tau_0$  at the water surface so accurately as to be able to examine the validity of the relation (14). In the boundary layer over the waves, mean wind profiles can not be measured near the water surface. Even if the wind speed is measured, it is often less accurate because of the large velocity fluctuations than over the solid surface. Moreover it has often been pointed out or discussed that the wind speeds deviate from the log-profile (14) near the water surface (e.g. Stewart (1961), Hamada *et al.* (1963), Mitsuyasu (1964), Phillips (1966), Hamada (1968), etc.). For these reasons there remains the following question to be solved, even if the log-law is applicable to the wind profiles over waves: To what portion of the profiles should the relation (14) be applied to obtain the real  $u_*$  (or  $\tau_0$ )?

Notwithstanding these problems the relation (14) has often been used in order to estimate  $u_*$  from the wind profiles (but occasionally in conflicting ways), since  $u_*$  is a representative parameter in the experiment of wind and waves and this method is usually the only simple means to obtain it.

One of the main object of the present experiment was to shed some light on the applicability of the log-law to the wind profiles over water waves. This could not be accomplished satisfactorily because of the seasonally inconsistent results in the values of  $-\overline{u'w'}$  described before. However we will discuss to the possible extent about the relevant matters mainly on the assumption that the results obtained at the summer season are less reliable than those at the other seasons in the present experiment.

As for the theoretical base of the log-law, it is derived from the mixing-length theory of Prandtl or Karman (e.g. Schlichting (1968)) by presuming the existence of the constant-stress layer next to the boundary but outside the viscous sub-layer, although there are also other derivations from the similarity reasoning (Townsend (1956), Hinze (1959), etc.).

The values of  $-\overline{u'w'}$  obtained at the seasons other than summer for each run of both rpm 200 and rpm 300 increased with decreasing height until a certain height  $z_m$  where they took the maximum values, and  $z_m \doteq 5 \sim 6$  cm for rpm 200 and  $z_m \doteq 8 \sim 10$  cm for rpm 300 (see Figs. 20 and 21). Below those heights  $-\overline{u'w'}$  decreased toward the water surface. However the reduction of  $-\overline{u'w'}$  near the wave crests were mostly attributed to the wave-induced Reynolds stress  $-\rho_a \overline{u'w'}$  as mentioned previously. Taking this into consideration, the turbulent Reynolds stress distribution is regarded as nearly constant over the short distances above the wave crests. Then the wind shear stress acting on the water surface  $\tau_0$  is expected to have approximately the same values as the turbulent Reynolds stresses  $-\rho_a \overline{u'w'}$  attained there, which are in turn practically equal to the values of  $-\rho_a \overline{u'w'}$  at the height  $z_m$  mentioned above. In this way,  $u_*^2$  is expected to be equal to  $-\overline{u'w'}$  at the height  $z_m$ .

The measurements of the mean wind profiles for rpm 200 were made several times throughout a year similarly to the measurements of velocity fluctuations. Two of those profiles which were measured in winter are shown in Fig. 33-a, b. The profile in Fig. 33-a has a good linear portion, which is indicated by a line there. Applying the relation (14) to this portion, we obtain  $u_* = 26.6$  cm/sec and consequently  $u_*^2 = 708$  cm<sup>2</sup>/sec<sup>2</sup>. On the other hand, the profile in Fig. 33-b has too much scatter for one to find a suitable straight line for the log-law. It was not rare in our experiments that the scatter of the points in the obtained wind

profiles was as much as that in Fig. 33-b. If we dare to draw a line such as seen in the figure (dotted one), it gives  $u_* = 22.6 \text{ cm/sec}$  and  $u_*^2 = 551 \text{ cm}^2/\text{sec}^2$ . Of course this value may include a considerable error, and the results will not be improved very much even if the least square error method is used. We presented the latter profile to illustrate the worst limit, but not rare in general, of the accuracy of  $u_*$  obtained from the wind profiles at the wind speed for rpm 200. Be that as it may, the values of  $u_*^2$  obtained from the two wind profiles are found to be in reasonably good agreement with the values of  $-\overline{u'w'}$  obtained at about  $z=5 \text{ cm}$  (see Fig. 20). Finally we make an additional remark that other two profiles with a good log-linear portion which were measured in summer gave a little larger values of  $u_*$ . However its relevance to the seasonal difference of  $-\overline{u'w'}$  is not clear.

On the other hand, the wind profiles for rpm 300 did not show a linear portion so clearly as for rpm 200. A typical wind profile for rpm 300 is shown in Fig. 34. If we attempt to apply the log-law to this kind of profile in order to obtain  $u_*$ , it is a problem which portion of the profile to take. One may take the lowest portion, drawing a line such as I in the figure, and another a little higher portion such as II (someone may even take the more higher portion). In the case of Fig. 34, the line I gave  $u_* = 35.1 \text{ cm/sec}$ ,  $u_*^2 = 1230 \text{ cm}^2/\text{sec}^2$ , while the line II gave  $u_* = 43.6 \text{ cm/sec}$ ,  $u_*^2 = 1900 \text{ cm}^2/\text{sec}^2$ . So long as we follow the assumption made earlier, the line I seems to give the better agreement of  $u_*^2$  obtained from the profile with the results of  $-\overline{u'w'}$  shown in Fig. 21.

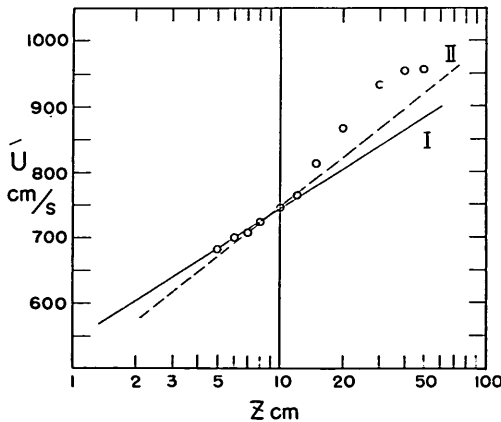


Fig. 34. Wind Velocity Profile in the case of wind waves only; rpm 300

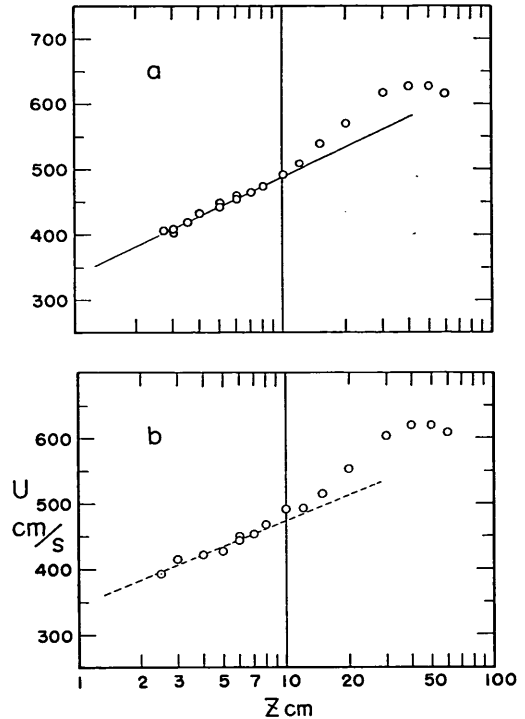


Fig. 33. Wind Velocity Profiles in the cases of wind waves only; rpm 200

### 3.3 The Case of Composite Long Waves

The experiment of this case was conducted mainly at the wind speed for rpm 200 ( $U_\infty = 6.35 \text{ m/sec}$ ) and less extensively than the other two cases. The waves were composed of predominant long waves of  $T_p = 1.60 \text{ seconds}$  and  $H = 5.7 \text{ cm}$  which were gen-

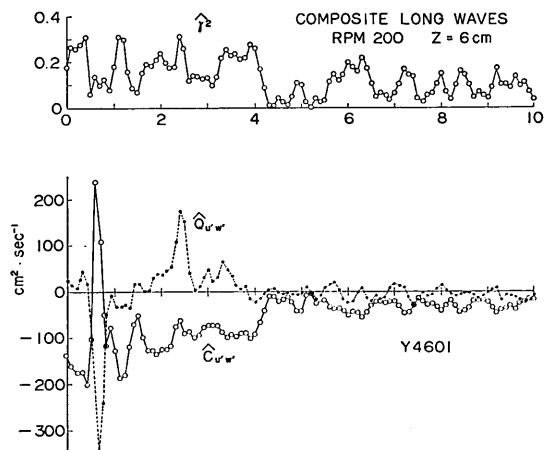


Fig. 35.  $u'w'$  Cross-Spectrum in the case of composite long waves; rpm 200,  $z=6$  cm

erated mechanically and smaller wind waves.

Co- and quad-spectra at  $z=6, 10$  and  $20$  cm, which were obtained in summer, are shown together with the coherences in Figs. 35~37. The results obtained in winter were not different from the results shown in those figures except that the cospectral densities in winter were slightly smaller than those in summer in accordance with the difference in  $-\overline{u'w'}$ , which will be shown later.

In this case the velocity fluctuations corresponding to the main frequency of wind waves ( $f \approx 2.5$  Hz) were not found very much either in  $u'$ - and  $w'$ -spectra or in co- and quad-spectra even at the lowermost measurement height ( $z=6$  cm). On the other hand, the fluctuations induced by the long waves appeared strongly just as in the cases of clear long waves with  $H=6.0$  or  $7.0$  cm. That is, the co-spectra obtained both in summer and in winter at all measurement heights had a strong positive peak near  $f = f_L \approx 0.6$  Hz, indicating that there existed a considerable amount of negative wave-induced Reynolds stress. In this case, however, no positive wave-induced stress was found, and this is probably because the

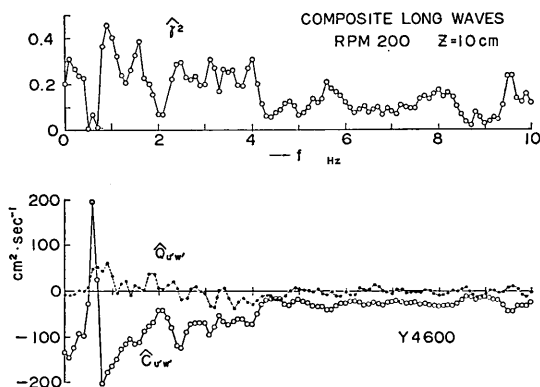


Fig. 36.  $u'w'$  Cross-Spectrum in the case of composite long waves; rpm 200,  $z=10$  cm

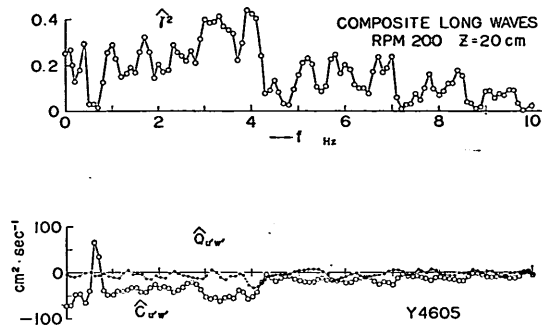


Fig. 37.  $u'w'$  Cross-Spectrum in the case of composite long waves; rpm 200,  $z=20$  cm

measurement could not be made so close to the mean wave crest as in the cases of clear long waves. One more notable matter is that the coherences  $\hat{\gamma}^2$  near  $f=0.6$  Hz were extremely small even at  $z=6$  cm and the obtained phase lag  $\hat{\theta}$  between  $u'$  and  $w'$  was consequently very unstable (according to Jenkins (1963), the variance of the estimates of phase lag is in proportion to  $[(1/\hat{\gamma}^2)-1]$ ). This is somewhat in contrast to the cases of clear long waves. At the height  $z=40$  cm, however, all turbulent components were remarkably reduced and alternatively the wave-induced components became prominent, producing a high  $u'w'$ -coherence of  $\hat{\gamma}^2=0.9$  at  $f=0.6$  Hz.

The measurements of  $u'$  and  $w'$  have also been made by Shemdin & Lai (1970) over the composite long waves ( $H=8.64$  cm,  $f_L=0.70$  Hz and  $c=226$  cm/sec) in a wind-wave tunnel a little larger than that used in our experiments by using hot-film anemometers, and the  $u'w'$ -cospectra were obtained by means of the analog method. Strange to say, however, there seems to be a definite difference between the results of Shemdin & Lai and ours with respect to the wave-induced Reynolds stresses. In their experiments the cospectrum for  $z=10.2$  cm obtained at the fetch 9.15 m for the reference wind speed 10.7 m/sec exhibited a very dominant negative peak at the frequency of the mechanically generated waves  $f_L=0.70$  Hz, indicating a large positive wave-induced Reynolds stress. That positive stress distinctly appeared also at the heights of  $z=20.3$  cm and  $z=50.8$  cm although their absolute values decreased with increasing height. The positive wave-induced stresses were also found at the fetch 25.4 m for the same wind speed.

For the purpose of reference, we made measurements of  $u'w'$ -cospectra for the higher wind speeds, such as  $U_\infty=8.0, 9.6, 11.3, 12.9$  m/sec at the height  $z=10$  cm, but only the negative wave-induced Reynolds stresses similar to those in Figs. 35 and 36 were found.

Our results were obtained at the fetch 18.8 m, while those of Shemdin & Lai at the fetch 9.15 m and 25.4 m. It is least probable, however, that the wave-induced Reynolds stresses vary changing their sign alternately with fetch. Then the reason can not be made out why our results differed so seriously from those of Shemdin & Lai (1970).

The values of  $-\overline{u'w'}$  obtained from the two different runs conducted in summer and winter, respectively, are shown in Fig. 38 (with dark marks) together with the corresponding results for the cases of wind waves only obtained at the same day (with open marks). The difference of the results between the two

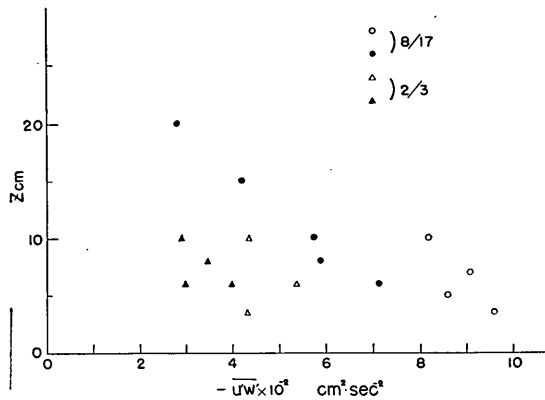


Fig. 38. Distribution of  $-\overline{u'w'}$  (rpm 200): ●, ▲—composite long waves; ○, △—wind waves only

seasons is obvious and just the same as in the cases of wind waves only. It is further seen that the values of  $-\overline{u'w'}$  for the cases of composite long waves are considerably smaller than those for wind waves only. Of course the negative wave-induced Reynolds stresses existed in the former cases, but their magnitude roughly estimated from the variations of cospectra by the graphical method of Fig. 10 are  $60\sim 70$  cm<sup>2</sup>/sec<sup>2</sup> at most and obviously smaller than the above-mentioned difference between the former and the latter cases. Therefore, the turbulent Reynolds stresses in the former cases seem to be smaller than those in the latter cases at the small heights above the water surface.

#### 4. Conclusions

The measurements have been made of the horizontal and vertical wind velocity fluctuations,  $u'$  and  $w'$ , over the three kind of waves in a wind-wave tunnel by the use of an X-array hot-wire system and the digital method. The main findings obtained are as follows:

1) The existence of the wave-induced velocity fluctuations was found not only in  $u'$ - and  $w'$ -spectra but also in the cross spectra between  $u'$  and  $w'$ .

2) It was found that  $-\overline{u'w'}$  obtained from the variation of cospectra by the graphical method of Fig. 10 gives a reasonably good estimate of the wave-induced Reynolds stresses.

3) In the cases of 'clear long waves', the wave-induced Reynolds stresses definitely existed above the critical layer. When the wave height was small ( $H=1.1$  cm), only the positive wave-induced stresses were found at the small height above the wave crests. For the larger wave heights ( $H=6.0$  or  $7.0$  cm), however, the negative wave-induced stresses were prevailing over most of the height except slightly above the wave crests where they were positive.

4) In the cases of wind waves only, the wave-induced velocity fluctuations also existed considerably at the lowest height above the wave crests. However they were almost  $90^\circ$  out of phase and did not make much contribution to the  $u'w'$ -cospectra if any, mostly contributing to the quad-spectra. Nevertheless the cospectra obtained in winter showed signs of the negative wave-induced stresses.

5) In these cases the seasonal differences, not only quantitative but also



qualitative, were found in the values of  $-\overline{u'w'}$  especially near the water surface. The results obtained at the seasons other than summer indicated that the turbulent Reynolds stresses are likely to be nearly constant at the small heights above the wave crests. It was further found that those nearly constant stresses are in reasonable agreement with the surface shear stresses estimated by applying the log-law to the lowest portion of the wind velocity profiles.

6) In the cases of composite long waves, only the negative wave-induced Reynolds stresses of non-negligible magnitude were found at all the measurement heights regardless of the season, and the turbulent velocity fluctuations were predominant even at the lowest height somewhat in contrast to the cases of 'clear long waves'.

The authors wish to express their appreciation to Dr. T. Hamada, Chief of Hydraulic Laboratory, for the suggestion of the measurement for the case of smaller wave height and for the criticisms given in the course of preparation of this report. The digital computations have been conducted by the use of a digital computer TOSBAC 3400 at the Computation Center of the Port and Harbour Research Institute, and the thanks are also due to Messrs. N. Ogawa and M. Tsubata, members of the Computation Center, for the arrangements offered to the authors mainly in the connection of the digital data recorder with the computer.

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### List of Symbols

- $a_1$ : hot-wire constant (cf. Eq. 1)  
 $c$ : phase velocity of periodic waves  
 $E$ : output voltage from a hot-wire anemometer  
 $E_0$ : hot-wire constant (cf. Eq. 1)  
 $\hat{C}_{u'w'}(f)$ : cospectral density between  $u'$  and  $w'$   
 $f$ : frequency  
 $f_L$ : frequency of the mechanically generated waves  
 $f_N$ : holding frequency in the spectral computations  
 $f_p$ : frequency at which a wave spectrum shows the maximum density  
 $\Delta f$ : frequency resolution in the spectral computations  
 $h$ : maximum lag in the spectral computations  
 $H$ : crest-to-trough wave height  
 $H_{1/3}$ : significant wave height estimated from the total power of the wave spectra  
 $K$ : conversion coefficient (cf. Eq. 2)  
 $M$ : total output voltage from a linearizer  
 $N$ : total number of the data used in a computation  
 $\hat{Q}_{u'w'}(f)$ : quad-spectral density between  $u'$  and  $w'$   
 $r_a(f)$ : attenuation factor for the 3-point array  
 $r_s(f)$ : attenuation factor for the 3-point sliding mean  
rpm: revolution number of the blower's fan per minute  
 $t$ : time position corresponding to a particular wave phase  
 $\Delta t$ : data sampling time interval  
 $T_p$ : wave period  
 $\Delta T$ : temperature difference between air (in the laboratory) and water  
 $\Delta T_1$ : temperature difference of flowing air between near the water surface and at  $z=40$  cm  
 $u'$ : total horizontal (streamwise) velocity fluctuations ( $=\bar{u}+u'_i$ )  
 $\bar{u}$ : horizontal wave-induced velocity fluctuations ( $=\langle u' \rangle$ )  
 $u'_i$ : horizontal turbulent velocity fluctuations  
 $u_*$ : friction velocity ( $=\sqrt{\tau_0/\rho_a}$ )  
 $U$ : mean wind speed  
 $U_\infty$ : maximum wind speed  
 $w'$ : total vertical velocity fluctuations ( $=\bar{w}+w'_i$ )  
 $\bar{w}$ : vertical wave-induced velocity fluctuations ( $=\langle w' \rangle$ )  
 $w'_i$ : vertical turbulent velocity fluctuations  
 $V$ : normal wind speed to a hot-wire  
 $-\bar{u}\bar{w}$ : wave-induced Reynolds stress divided by  $\rho_a$   
 $-\overline{u'_i w'_i}$ : turbulent Reynolds stress divided by  $\rho_a$   
 $X, Y$ : total output voltages from an X-array hot-wire system,  $X$  corresponding to a wire A in Fig. 2  
 $X', Y'$ : fluctuating part of  $X$  and  $Y$ , respectively  
 $z$ : height above the mean water surface  
 $z_m$ : height at which  $-\overline{u'w'}$  takes the maximum value  
 $z_0$ : roughness length  
 $—$ : upper bar denoting overall time average

- $\langle \rangle$ : time average with respect to a particular wave phase
- $\langle \rangle_*$ : equivalent to  $\langle \rangle$ .
- $\alpha', \beta'$ : constants determined from the hot-wire calibration curves
- $\alpha, \beta$ : constants ( $\alpha = \alpha' / \sqrt{2}$ ,  $\beta = \beta' / \sqrt{2}$ )
- $\hat{r}^2$ : coherence between  $u'$  and  $w'$
- $\varepsilon$ : subscript referring to a particular wave phase
- $\kappa$ : Karman's constant ( $\doteq 0.4$ )
- $\hat{\theta}$ : phase lag of the  $w'$ -data relative to the  $u'$ -data
- $\rho_a$ : density of air
- $\tau_0$ : wind shear stress at the water surface
- $\Delta\tau$ : original sampling time interval for the 3-point sliding mean

Appendix

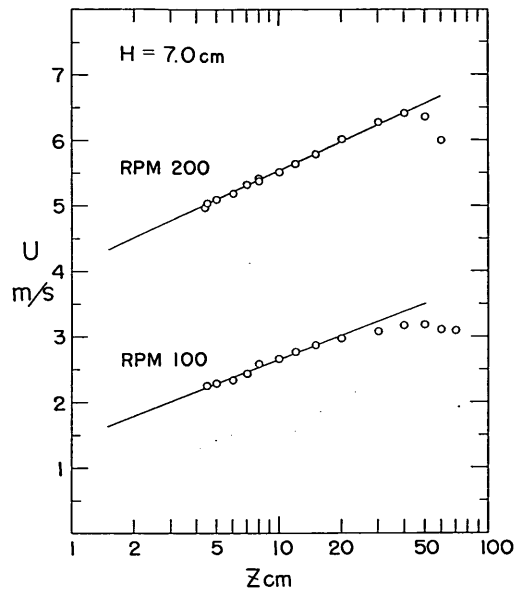


Fig. A-1. Wind Velocity Profiles in the cases of clear long waves; rpm 200 & 100,  $H=7.0$  cm

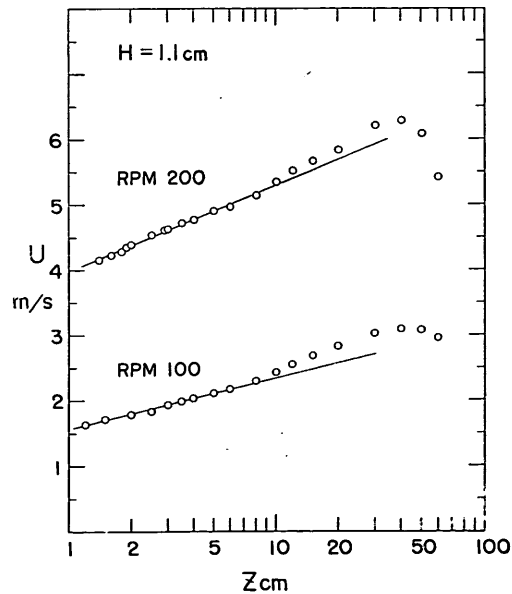


Fig. A-2. Wind Velocity Profiles in the cases of clear long waves; rpm 200 & 100,  $H=1.14$  cm

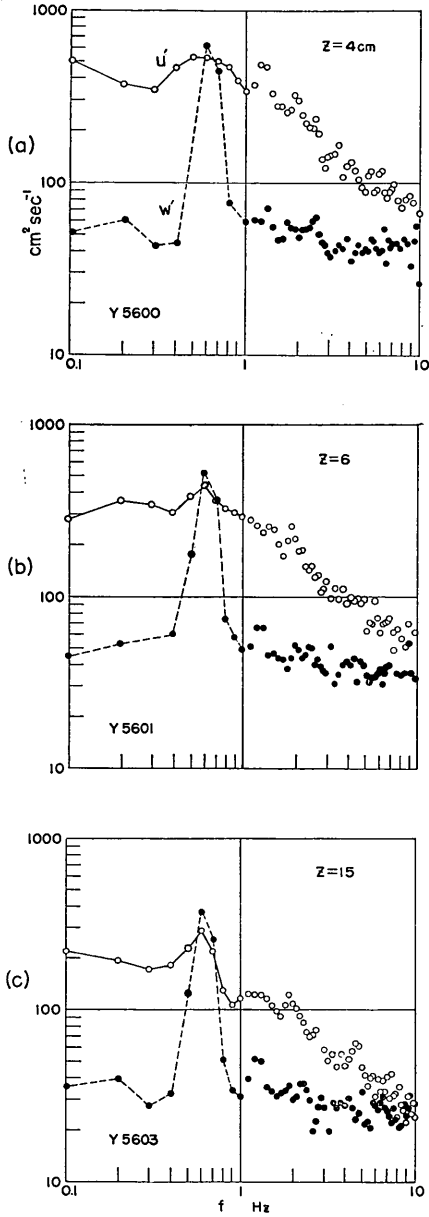


Fig. A-3.  $u'$ - and  $w'$ -Spectra in the cases of clear long waves; rpm 200,  $H=6.0$  cm

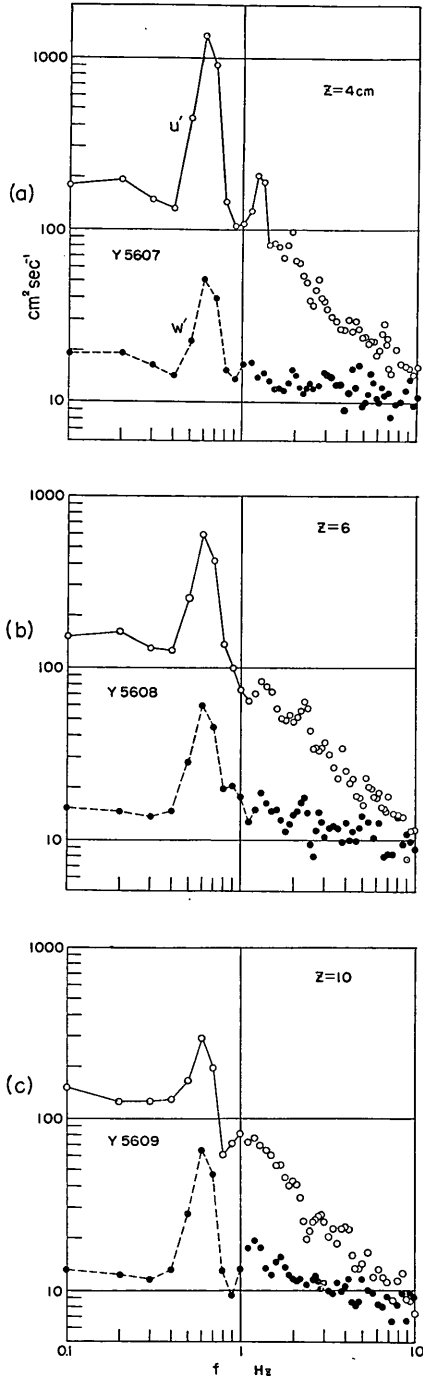


Fig. A-4.  $u'$ - and  $w'$ -Spectra in the cases of clear long waves; rpm 100,  $H=6.0$  cm

An Experimental Study of the Turbulent Structure of Wind over Water Waves

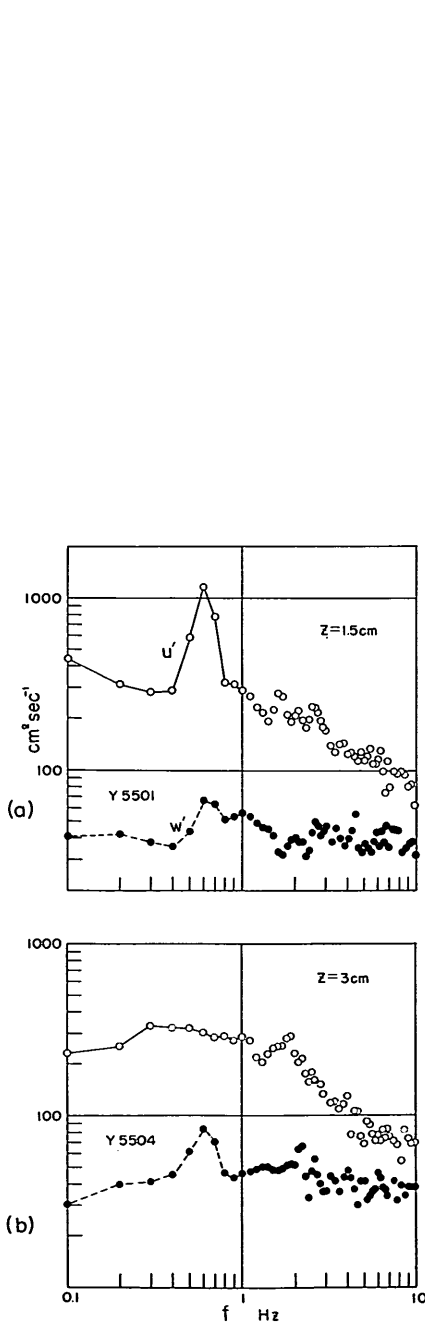


Fig. A-5.  $u'$ - and  $w'$ -Spectra in the cases of clear long waves; rpm 200,  $H=1.14$  cm

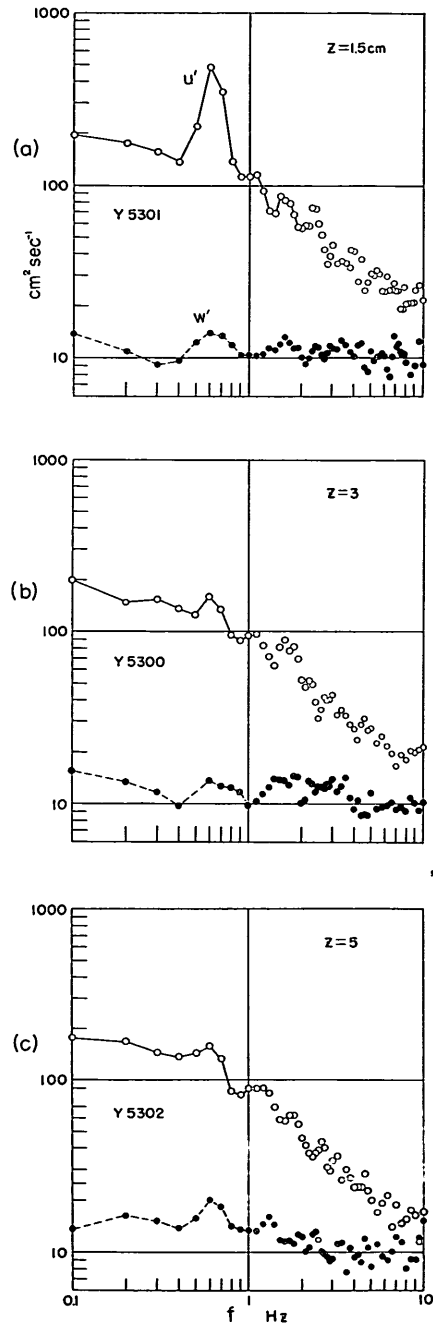


Fig. A-6.  $u'$ - and  $w'$ -Spectra in the cases of clear long waves; rpm 100,  $H=1.14$  cm

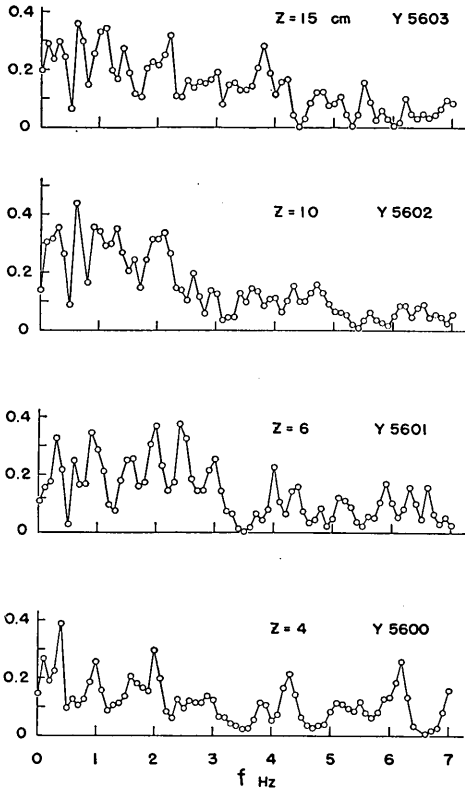


Fig. A-7.  $u'w'$  Coherences for the cases of Fig. 5 (rpm 200 &  $H=6.0$  cm)

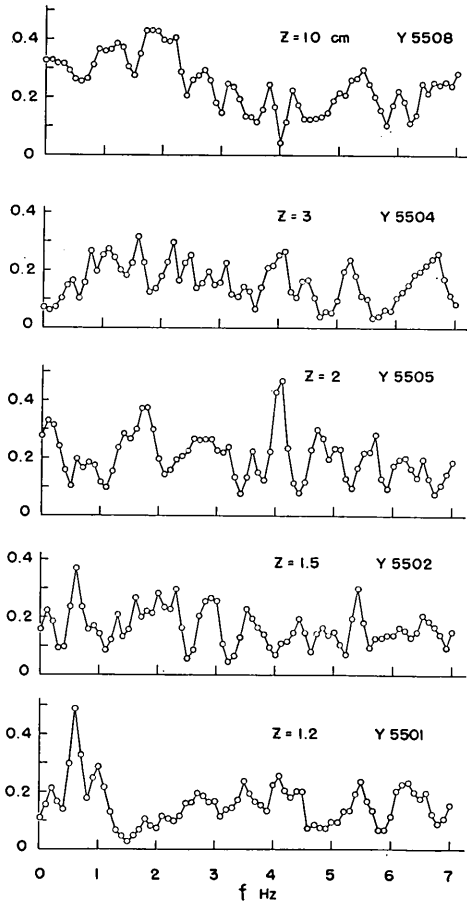


Fig. A-8.  $u'w'$  Coherences for the cases of Fig. 6 (rpm 200 &  $H=1.14$  cm)



An Experimental Study of the Turbulent Structure of Wind over Water Waves

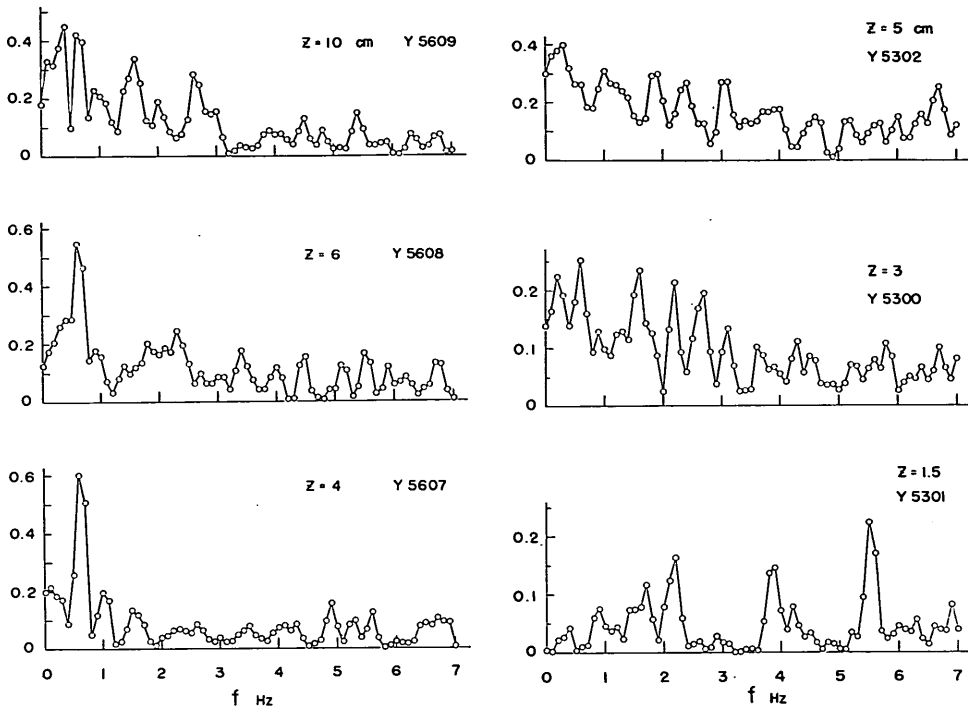


Fig. A-9.  $u'w'$  Coherences for the cases of Fig. 7 (rpm 100 &  $H=6.0$  cm)

Fig. A-10.  $u'w'$  Coherences for the cases of Fig. 8 (rpm 100 &  $H=1.14$  cm)

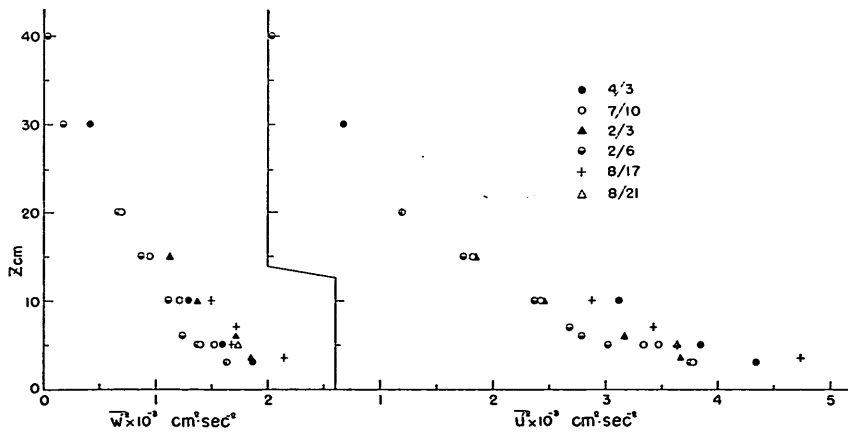


Fig. A-11. Distributions of  $\overline{w'^2}$  and  $\overline{u'^2}$  in the cases of wind waves only, rpm 200

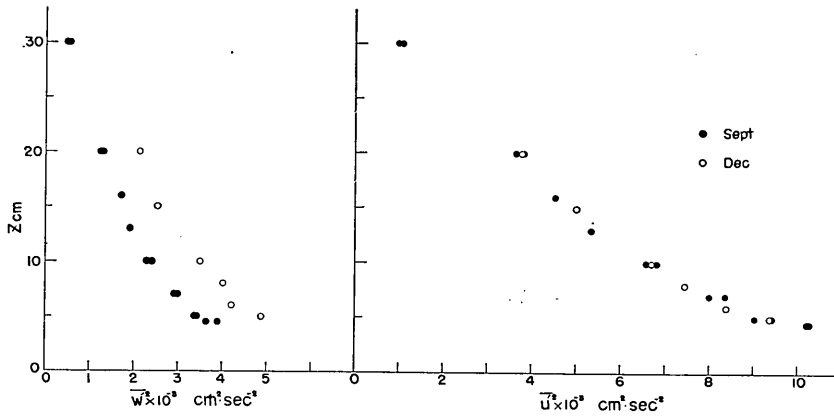


Fig. A-12. Distributions of  $\overline{w'^2}$  and  $\overline{u'^2}$  in the cases of wind waves only, rpm 300

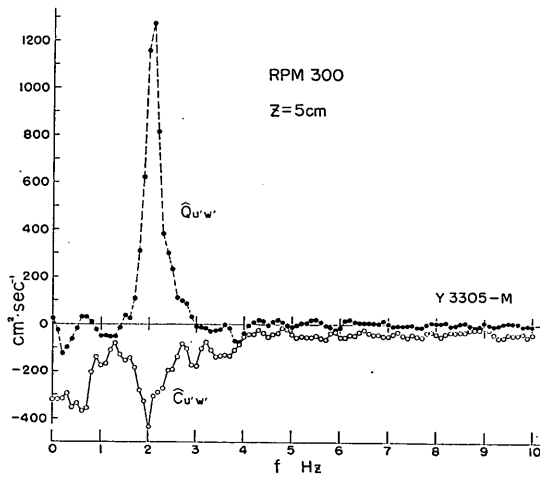


Fig. A-13. Co- and Quad-Spectrum in the case of wind waves only, at  $z=5\text{ cm}$ , rpm 300